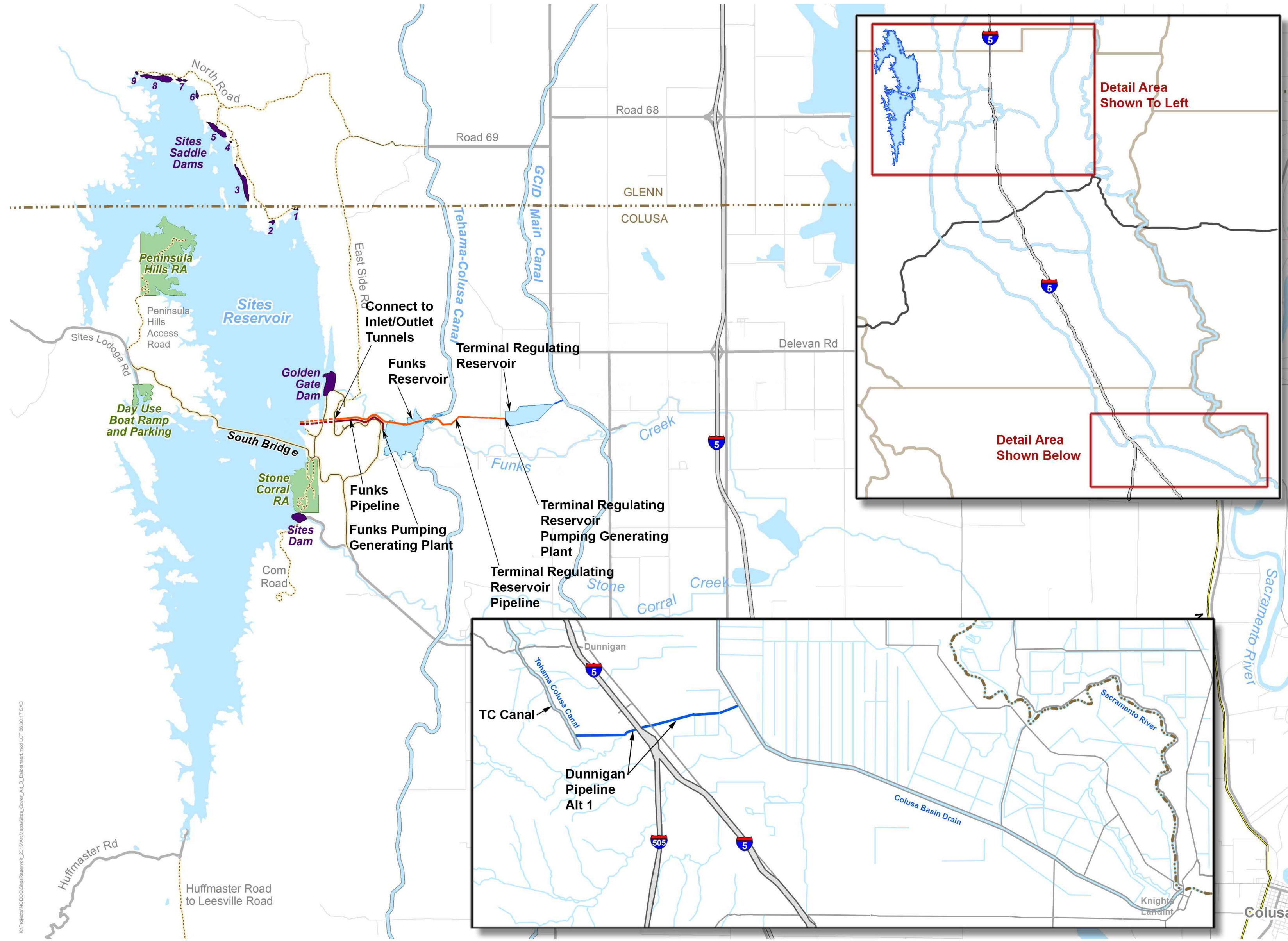


SITES RESERVOIR MAXWELL / SITES PUMPING AND GENERATING PROJECT FUNKS RESERVOIR 30% DESIGN - CLIENT REVIEW DECEMBER 19, 2023



PROJECT LOCATION MAP



OVERALL PROJECT SITE MAP - DUNNIGAN PIPELINE NOT INCLUDED IN THIS PACKAGE

Plot Date: 12/18/2023 4:24 PM File: C:\pwworking\hdr_sites_reservoir\dms02763\MPG-0001-G-0001_FNK_RES.dwg Saved By: DCAVE

REV	DATE	BY	CHK.	APPR.	DESCRIPTION

DESIGNED BY:
D. CAVE
DRAWN BY:
D. CAVE
CHECKED BY:
W. OHLIN
IN CHARGE:
P. RUDE
DATE:
12-19-2023

Jacobs
2525 AIRPARK DR
REDDING, CA 96001
(530) 243-5831

Geosyntec
consultants

REGISTERED
PROFESSIONAL
ENGINEER
WAYNE J. OHLIN
72287
CALIFORNIA



SITES RESERVOIR
MAXWELL / SITES PUMPING AND GENERATING
GENERAL
COVER SHEET,
LOCATION MAP AND SITE MAP

VERIFY SCALES
BAR IS ONE INCH ON ORIGINAL
DRAWING. ADJUST SCALES FOR
REDUCED PLOTS
0 1"
DRAWING NO.
MPG-0001-G-0001
SHT 1 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

INDEX OF DRAWINGS

SHEET NUMBER	DRAWING NUMBER	TITLE
1	MPG-0001-G-0001	COVER SHEET, LOCATION MAP AND SITE MAP
2	MPG-0001-G-0002	INDEX OF DRAWINGS
3	MPG-0001-G-0010	ABBREVIATIONS
4	MPG-0001-G-0020	GENERAL SYMBOLS AND DRAWING NUMBERING LEGEND
5	MPG-0001-G-0101	GENERAL CIVIL LEGEND
6	MPG-0001-G-0301	GENERAL STRUCTURAL NOTES 1
7	MPG-0001-G-0302	GENERAL STRUCTURAL NOTES 2
8	MPG-0045-C-2001	OVERALL LOCATION AND SURVEY CONTROL PLAN
9	MPG-2115-C-2101	FUNKS RESERVOIR - STOCKPILE AND HAUL ROUTE - PLAN
10	MPG-2115-C-2102	FUNKS RESERVOIR - RESERVOIR EXCAVATION STOCKPILES AND IMPOUNDMENT - PLAN
11	MPG-2115-C-2103	FUNKS RESERVOIR - WEST EXCAVATION - PLAN
12	MPG-2115-C-2104	FUNKS RESERVOIR - EAST EXCAVATION - PLAN
13	MPG-2115-C-2105	FUNKS RESERVOIR - PGP COFFERDAM - PLAN AND PROFILE
14	MPG-2115-C-2106	FUNKS RESERVOIR - PGP APPROACH CHANNEL - PLAN
15	MPG-2115-C-2107	FUNKS RESERVOIR - PGP COFFERDAM - SECTION AND DETAILS
16	MPG-2115-C-2108	FUNKS RESERVOIR - RESERVOIR EXCAVATION - PROFILES
17	MPG-2115-C-2109	FUNKS RESERVOIR - PGP APPROACH CHANNEL - PROFILES
18	MPG-2115-C-3410	FUNKS RESERVOIR - PGP APPROACH CHANNEL - DETAILS
19	MPG-2115-C-3411	FUNKS RESERVOIR - SEDIMENT IMPOUNDMENT - PLAN AND PROFILE
20	MPG-2115-C-3412	FUNKS RESERVOIR - SEDIMENT IMPOUNDMENT BERM - PROFILE AND DETAILS

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 Saved By: SKHALAMYZER

REV	DATE	BY	CHK	APPR.	DESCRIPTION

DESIGNED BY: S. KHALAMYZER
 DRAWN BY: S. KHALAMYZER
 CHECKED BY: B. MARTINEZ
 IN CHARGE: P. RUDE
 DATE: 12-19-2023



REGISTERED
 PROFESSIONAL
 ENGINEER

 CALIFORNIA



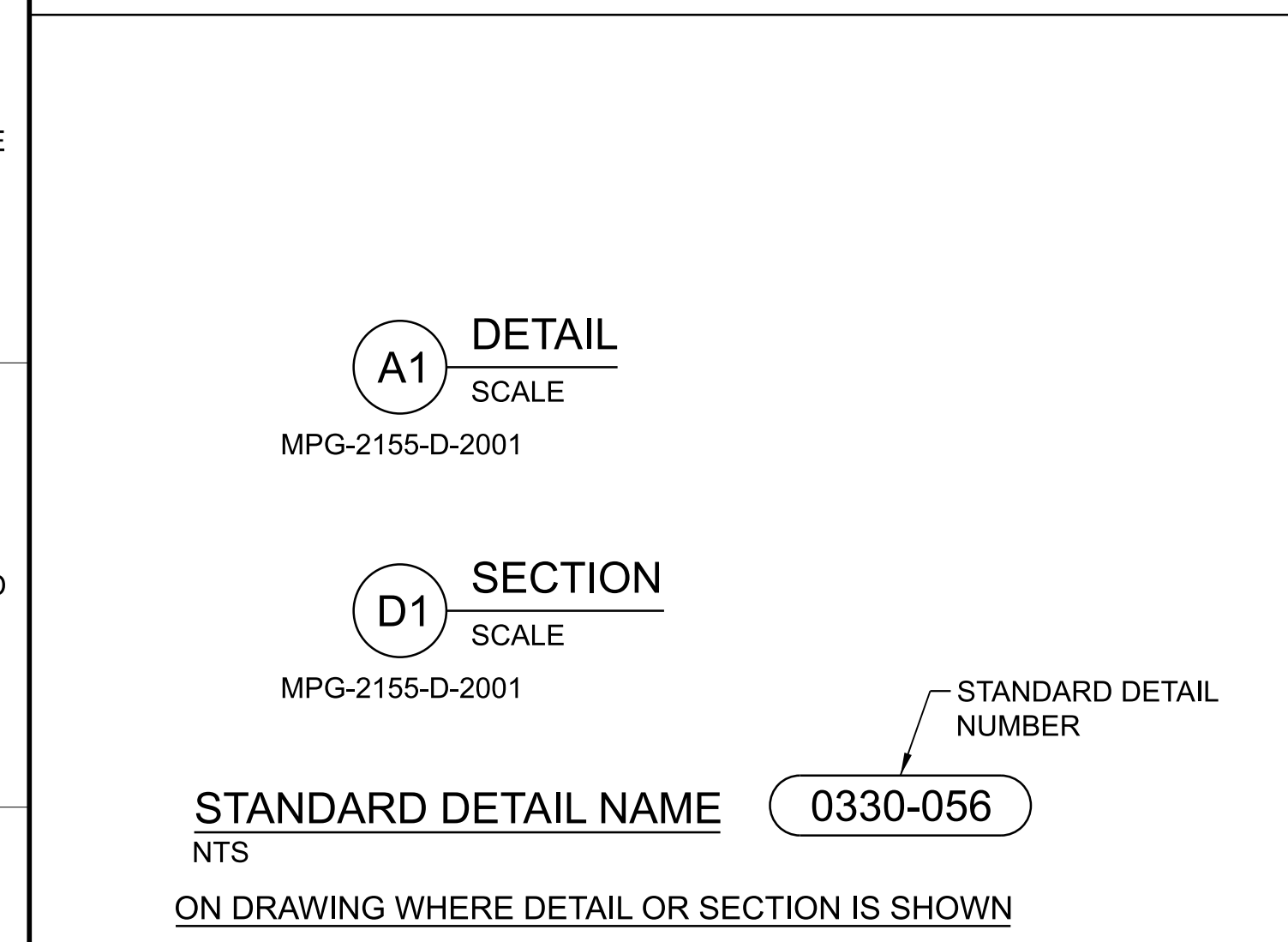
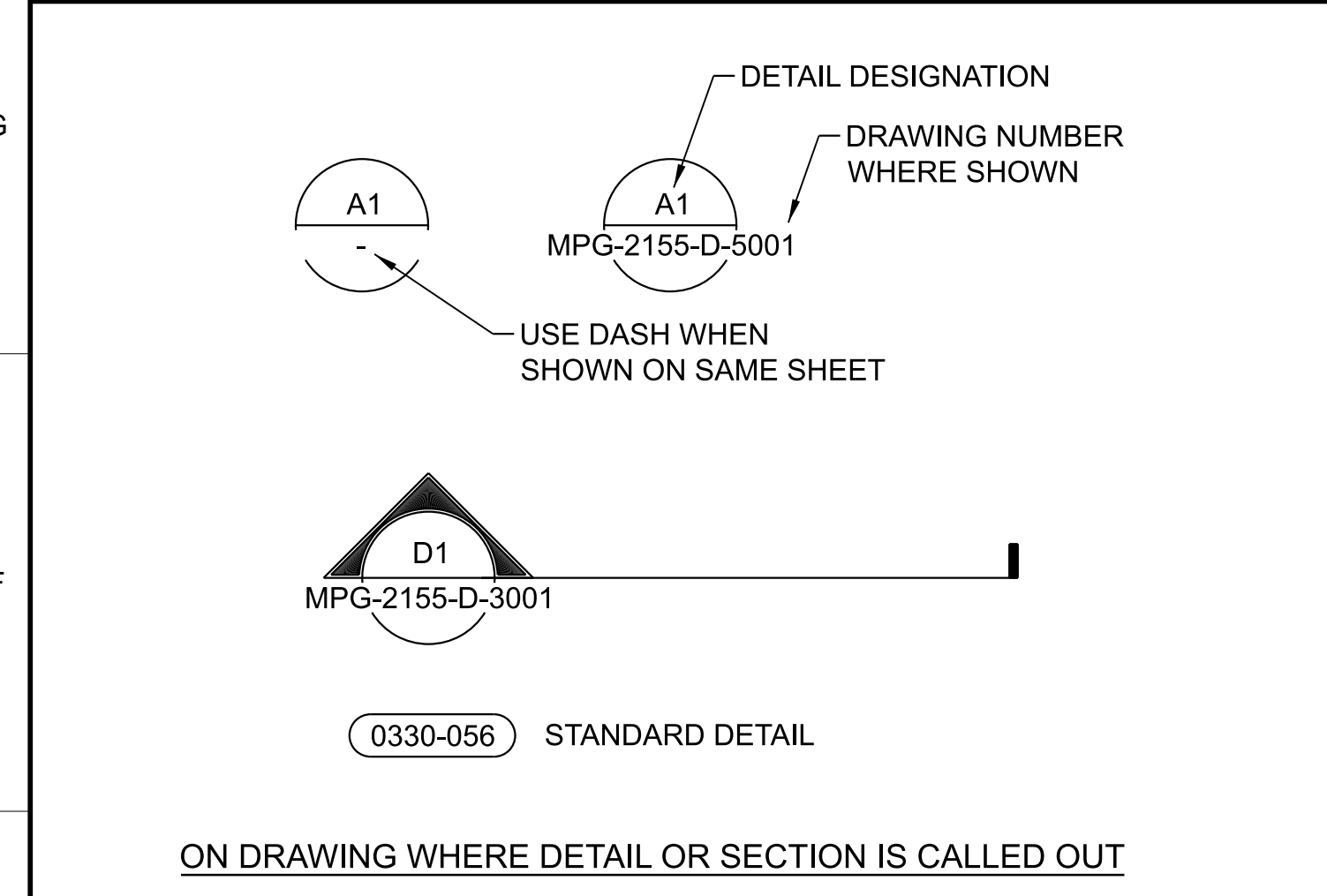
SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING
 GENERAL
 FUNKS RESERVOIR
 INDEX OF DRAWINGS

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 DRAWING. ADJUST SCALES FOR
 REDUCED PLOTS
 0 1"
 DRAWING NO.
 MPG-0001-G-0002
 SHT 2 OF 20

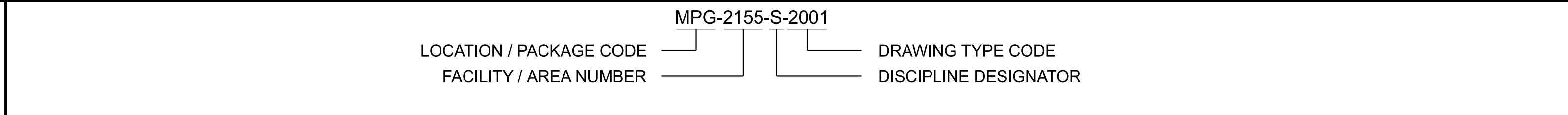
PRELIMINARY - NOT FOR CONSTRUCTION

Plot Date: 11/14/2022 1:08 PM File: C:\pwworking\hdr_sites_reservoir\dms00728\SITES-BRDR_22X34.dwg Saved By: AJACKSON

GENERAL SYMBOLS



DRAWING NUMBERING LEGEND



LOCATION / PACKAGE NUMBER AND CODE	MPG FACILITY / AREA NUMBERS	DISCIPLINE DESIGNATOR	DRAWING TYPE CODE
1- STS - SITES RESERVOIR 2- MPG - MAXWELL / SITES PUMPING AND GENERATING 3- SCD - RESERVOIR CLEARING AND DEMOLITION 4- HFR - HUFFMASTER ROAD 5- DNP - DUNNIGAN PIPELINE 6- CCA - TEHAMA-COLUSA CANAL AUTHORITY 7- CID - GLENN-COLUSA IRRIGATION DISTRICT 8- REC - SITES RECREATION 9- MIT - SITES MITIGATION	0001 - GENERAL 0010 - GEOTECH 0045 - OVERALL SITE CIVIL 0060 - OVERALL SITE ELECTRICAL 0065 - INSTRUMENTATION AND CONTROLS 2005 - ACCESS ROADS 2010 - TRANSMISSION 2015 - PGE POI SWITCHYARD 2030 - VALVE VAULT 2040 - ENVIRONMENTAL WATER PIPELINE DISSIPATION STRUCTURE 2100 - FNK - TEMPORARY CONSTRUCTION 2105 - FNK - SITE CIVIL 2107 - FNK - RETAINING WALL 2110 - FNK - YARD PIPING 2115 - FNK - RESERVOIR 2120 - FNK - PIPELINE 2125 - FNK - SITE AND GENERAL ELECTRICAL 2130 - FNK - SUBSTATION 2135 - FNK - INSTRUMENTATION AND CONTROLS 2145 - FNK - ADMINISTRATION AND OPERATIONS BUILDING 2150 - FNK - MAINTENANCE AND STORAGE BUILDING 2155 - FNK - PUMPING PLANT 2160 - FNK - SWITCHGEAR BUILDING 2161 - FNK - EMERGENCY GENERATOR 2165 - FNK - GENERATING PLANT 2170 - FNK - CHILLER YARD 2171 - FNK - HVAC BUILDING 2175 - FNK - EMERGENCY DISSIPATION STRUCTURE 2180 - FNK - FIRE WATER TANK 2181 - FNK - FIRE WATER PUMPING PLANT 2185 - FNK - SURGE CONTROL SYSTEM 2190 - FNK - FLOW METER VAULT 2200 - TRR - TEMPORARY CONSTRUCTION 2205 - TRR - SITE CIVIL 2207 - TRR - SHEET PILE WALL 2210 - TRR - YARD PIPING 2215 - TRR - RESERVOIR 2220 - TRR - PIPELINE 2225 - TRR - SITE ELECTRICAL 2230 - TRR - SWITCHYARD 2231 - TRR - SUBSTATION 2235 - TRR - INSTRUMENTATION AND CONTROL 2240 - TRR - TRANSMISSION 2255 - TRR - PUMPING PLANT 2260 - TRR - SWITCHGEAR BUILDING 2261 - TRR - EMERGENCY GENERATOR 2265 - TRR - GENERATING PLANT 2270 - TRR - CHILLER YARD 2271 - TRR - HVAC BUILDING 2275 - TRR - ENERGY DISSIPATION STRUCTURE 2280 - TRR - FIRE WATER TANK 2281 - TRR - FIRE WATER PUMPING PLANT 2285 - TRR - SURGE CONTROL SYSTEM 2291 - TRR - CHECK STRUCTURE 1 2292 - TRR - CHECK STRUCTURE 2 2293 - TRR - CHECK STRUCTURE 3 2294 - TRR - CHECK STRUCTURE 4	A - ARCHITECTURAL B - GEOTECHNICAL C - CIVIL D - PROCESS MECHANICAL E - ELECTRICAL F - FIRE PROTECTION FET - FOUNDATION EXCAVATION AND TREATMENT G - GENERAL H - HVAC I - INTERIORS J - PLUMBING K - TRANSMISSION L - LANDSCAPE M - BUILDING MECHANICAL N - INSTRUMENTATION AND CONTROLS P - PIPELINE Q - EQUIPMENT R - ROADWAY S - STRUCTURAL T - TELECOMMUNICATIONS V - SURVEY MAPPING Y - YARD PIPING	0000 - GENERAL AND 3D RENDERINGS 1000 - DEMOLITION 2000 - PLANS AND PLAN AND PROFILE 3000 - SECTIONS, ELEVATIONS AND PROFILES 4000 - ENLARGED PLANS 5000 - DETAILS 6000 - SCHEDULES AND DIAGRAMS 7000 - USER DEFINED 8000 - USER DEFINED 9000 - STD DETAILS

REV	DATE	BY	CHK.	APPR.	DESCRIPTION

DESIGNED BY: D. CAVE
 DRAWN BY: D. CAVE
 CHECKED BY: W. OHLIN
 IN CHARGE: P. RUDE
 DATE: 12-19-2023




2525 AIRPARK DR
 REDDING, CA 96001
 (530) 243-5831

REGISTERED PROFESSIONAL ENGINEER
 WAYNE J. OHLIN
 72287
 CALIFORNIA



SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING
 GENERAL
 GENERAL SYMBOLS
 AND DRAWING NUMBERING LEGEND

VERIFY SCALES
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 DRAWING NO.
 MPG-0001-G-0020
 SHT 4 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

LEGEND AND SYMBOLS

EXISTING FEATURES

- x—x— DRAIN ROCK
- x—x— FENCE
- x—x— SUBSTATION FENCE
- x—x— GATE
- 560— MAJOR CONTOURS
- 560— MINOR CONTOURS
- ○ ○ SILO(S), TANK(S)
- p ○ TRAFFIC SIGN
- ● ○ FIRE HYDRANT
- POST
- ○ ○ TREE
- ○ ○ PALM TREE
- ○ ○ BUSH
- ○ ○ POLE
- △ ○ SURVEY CONTROL POINT
- ○ ○ MANHOLE
- ○ ○ MISC UTILITY
- ○ ○ UTILITY BOX
- ○ ○ TRAFFIC LIGHT
- ○ ○ UTILITY JUNCTION
- ○ ○ BILLBOARD
- ○ ○ CATCH BASIN, RECT
- ○ ○ VA-TRAF-BARR-POST
- ○ ○ COMMUNICATION ANTENNA
- ○ ○ UTILITY VALVE
- ○ ○ SIGN, REFLECTIVE
- ○ ○ MAILBOX
- ○ ○ STORM DRAIN INLET
- — — ROAD, CENTER
- — — ROAD, ALIGNMENT
- — — ROAD
- — — DRIVEWAY
- — — BUILDING OUTLINE
- — — WALL, RETAINING WALL
- — — WALL, RETAINING WALL WITH CONC BARRIER
- P-UNK — PIPE, UNIDENTIFIED
- — — HEADWALL
- — — CULVERT
- SAN — SANITARY SEWER UNDERGROUND PIPE
- SAN — SANITARY SEWER MANHOLE
- W — WATER UNDERGROUND PIPE
- G — NATURAL GAS UNDERGROUND PIPE
- FO — FIBER OPTIC LINE
- E — ELEC UNDERGROUND
- E-OVH — ELEC OVERHEAD
- ○ ○ POWER POLE
- ○ ○ GUY WIRE
- ○ ○ GUY ANCHOR
- ○ ○ TRANSMISSION TOWER, METAL
- — — CANAL
- ||||| RAILROAD
- ○ ○ — DITCH/FLOW LINE
- SD — STORM DRAIN UNDERGROUND PIPE
- SD — STORM DRAIN MANHOLE
- — — SLOPE BANK, CUT
- — — SLOPE BANK, FILL
- ⊗ 290.00 SPOT ELEVATION
- ⊙ BORE LOCATION & # BORE LOCATION AND NUMBER
- ⊙ TEST PIT LOCATION AND NUMBER
- ⊙ AUG - # DWR AUGER HOLE
- ⊙ LC - # DWR CORE HOLE
- ⊙ DH - # USBR CORE HOLE
- ⊙ XX GEOTECHNICAL BORING
- ⊙ XX STRUCTURE, BUILDING OR FACILITY
- ⊙ XX LOCATION POINT - COORDINATES

PROPOSED FEATURES

- ○ ○ DRAIN ROCK
- x—x— FENCE
- x—x— SUBSTATION FENCE
- x—x— GATE
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- ● ○ FIRE HYDRANT
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- ⊙ XX GEOTECHNICAL BORING
- ⊙ XX STRUCTURE, BUILDING OR FACILITY
- ⊙ XX LOCATION POINT - COORDINATES

EXISTING FEATURES

- TOP TOE EARTH SLOPE
- N/A STEEL CHECKER PLATE
- ||||| BENTONITE CEMENT GROUT
- ||||| BENTONITE PELLET SEAL
- CUTOFF WALL (DETAILS/SECTIONS)
- ORIGINAL GROUND
- AGGREGATE BASE
- DAM/LEVEE FILL
- ||||| DAM/LEVEE EMBANKMENT FILL
- FINE SAND
- CONCRETE
- CLSM
- ||||| ROCK SLOPE PROTECTION
- ||||| ASPHALT CONCRETE PAVEMENT
- ||||| GRAVEL SURFACING
- CUTOFF WALL (PLANS)
- — — LIMITS OF WORK
- — — BREAK LINE
- — — PIPE BREAK LINE
- — — CENTERLINE
- — — SPRING LINE CENTERLINE
- ||||| OR ||||| OR ||||| DEMOLITION
- OR ○ ———— STRUCTURE, BUILDING OR FACILITY
- EXISTING PIPE TO BE ABANDONED
- ||||| EXISTING PIPE TO BE DEMOLISHED
- CONSTRUCTION CONTRACT LIMIT
- CONSTRUCTION EASEMENT
- CABLE TV
- COMMUNICATION
- FIRE PROTECTION WATER SUPPLY
- GUARD RAIL
- PROPERTY LINE
- CONTRACTOR STAGING BOUNDARY
- RIGHT OF WAY
- SILT FENCE
- TELEPHONE OVERHEAD
- TELEPHONE UNDERGROUND
- TEMPORARY CONSTRUCTION EASEMENT
- PERMANENT EASEMENT
- CATV
- COMM
- F
- R/W
- SF
- T-OVH
- T
- TCE
- PE

EXISTING FEATURES

- SLOPE PERCENT OR RISE:RUN
- FLOW ARROW
- DIRECTION ARROW
- WATER SURFACE
- PIEZOMETER
- TOP TOE EARTH SLOPE
- N/A STEEL CHECKER PLATE
- ||||| BENTONITE CEMENT GROUT
- ||||| BENTONITE PELLET SEAL
- CUTOFF WALL (DETAILS/SECTIONS)
- ORIGINAL GROUND
- AGGREGATE BASE
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PROPOSED FEATURES

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- DIRECTION ARROW
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Plot Date: 12/11/2023 7:07 AM Saved By: DCAVE File: C:\pwworking\hdt_sites_reservoir\dms01711\MPG-0001-G-0101.dwg

REV	DATE	BY	CHK	APPR	DESCRIPTION

DESIGNED BY:	B. CHELONIS
DRAWN BY:	B. CHELONIS
CHECKED BY:	W. OHLIN
IN CHARGE:	P. RUDE
DATE:	12-19-2023

2525 AIRPARK DR
REDDING, CA 96001
(530) 243-5831

REGISTERED
PROFESSIONAL
ENGINEER
BECKY K. CHELONIS
C 59851
CALIFORNIA

SITES RESERVOIR
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DRAWING NO.
MPG-0001-G-0101
SHT 5 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

DESIGN CRITERIA

1. APPLICABLE CODE: 2022 CALIFORNIA BUILDING CODE (CBC) INCLUDING REFERENCED CODES AND STANDARDS.
2. REFER TO FACILITY DRAWINGS FOR ADDITIONAL AND SPECIFIC STRUCTURE LOADINGS AND REQUIREMENTS.
3. ALL LOADS SHOWN ARE SERVICE LEVEL (UNFACTORED) UNLESS SPECIFICALLY NOTED OTHERWISE.
4. DEAD LOADS: SELF WEIGHT
5. ROOF LOADS:
GROUND SNOW LOAD, Pg = 0 PSF
ROOF LIVE LOAD = 20 PSF
6. FLOOR LIVE LOADS:
PROCESS AREAS = 200 PSF
ELECTRICAL AREAS = 300 PSF
CORRIDORS, STAIRWAYS, ACCESS WAYS = 100 PSF
WALKWAYS AND ELEVATED PLATFORMS = 100 PSF
VEHICLE DRIVE AREAS = AASHTO DESIGN TRUCK OR DESIGN TANDEM
7. WIND LOADS:
ASCE 7-16 METHOD = MWFRS DIRECTIONAL PROCEDURE, UNO
BASIC WIND SPEED (3-SECOND GUST) = 104 MPH, RISK CATEGORY IV
BASIC WIND SPEED (3-SECOND GUST) = 100 MPH, RISK CATEGORY III
BASIC WIND SPEED (3-SECOND GUST) = 93 MPH, RISK CATEGORY II
EXPOSURE CATEGORY = C
8. FUNKS RESERVOIR SEISMIC LOADS:
MAPPED SPECTRAL RESPONSE ACCELERATIONS
S_s = 0.867g
S₁ = 0.359g
SITE-SPECIFIC DESIGN SPECTRAL RESPONSE ACCELERATIONS
S_{DS} = 0.TBDg
S_{D1} = 0.TBDg
SITE CLASS = D
SEISMIC DESIGN CATEGORY = D
9. TERMINAL REGULATING RESERVOIR SEISMIC LOADS:
MAPPED SPECTRAL RESPONSE ACCELERATIONS
S_s = 0.841g
S₁ = 0.350g
SITE-SPECIFIC DESIGN SPECTRAL RESPONSE ACCELERATIONS
S_{DS} = 0.TBDg
S_{D1} = 0.TBDg
SITE CLASS = D
SEISMIC DESIGN CATEGORY = D
10. RISK CATEGORY = SEE FACILITY DRAWINGS
11. IMPORTANCE FACTOR = SEE FACILITY DRAWINGS
12. LATERAL FORCE-RESISTING SYSTEM = SEE FACILITY DRAWINGS
13. FUNKS RESERVOIR SOIL DESIGN PARAMETERS:
NET ALLOWABLE SOIL BEARING PRESSURES: = TBD PSF (SHALLOW FOUNDATIONS)
GROUNDWATER (GW) ELEVATION:
MAXIMUM HIGH GW = EL 205.0
EQUIVALENT DRAINED FLUID PRESSURES:
ACTIVE: = TBD PCF
AT REST: = TBD PCF
PASSIVE: = TBD PCF
EQUIVALENT UNDRAINED FLUID PRESSURES:
ACTIVE: = TBD PCF
AT REST: = TBD PCF
PASSIVE: = TBD PCF
DYNAMIC FLUID PRESSURES:
YIELDING WALLS LATERAL FORCE: = TBD H LBS (APPLIED AT 0.6H)
NON-YIELDING WALLS: = TBD H PSF
WHERE H IS HEIGHT OF SOIL ADJACENT TO THE WALL
VERTICAL SURCHARGE: = EQUIVALENT 2 FT OF SOIL
COEFFICIENT OF FRICTION: = 0.TBD
MODULUS OF SUBGRADE REACTION = TBD PCI (1 FT SQUARE PLATE)
NATIVE SOIL UNIT WEIGHT = TBD PCF
MINIMUM FOOTING EMBEDMENT DEPTH: = TBD IN
14. TERMINAL REGULATING RESERVOIR SOIL DESIGN PARAMETERS:
NET ALLOWABLE SOIL BEARING PRESSURES: = TBD PSF (SHALLOW FOUNDATIONS)
GROUNDWATER (GW) ELEVATION:
MAXIMUM HIGH GW = EL 124.0
EQUIVALENT DRAINED FLUID PRESSURES:
ACTIVE: = TBD PCF
AT REST: = TBD PCF
PASSIVE: = TBD PCF
EQUIVALENT UNDRAINED FLUID PRESSURES:
ACTIVE: = TBD PCF
AT REST: = TBD PCF
PASSIVE: = TBD PCF
DYNAMIC FLUID PRESSURES:
YIELDING WALLS LATERAL FORCE: = TBD H² LBS (APPLIED AT 0.6H)
NON-YIELDING WALLS: = TBD H PSF
WHERE H IS HEIGHT OF SOIL ADJACENT TO THE WALL
VERTICAL SURCHARGE: = EQUIVALENT 2 FT OF SOIL
COEFFICIENT OF FRICTION: = 0.TBD
MODULUS OF SUBGRADE REACTION = TBD PCI (1 FT SQUARE PLATE)
NATIVE SOIL UNIT WEIGHT = TBD PCF
MINIMUM FOOTING EMBEDMENT DEPTH: = TBD IN
15. FACTOR OF SAFETY FOR BOUYANCY UPLIFT RESISTANCE:
NORMAL OPERATION FOS = 1.5 MINIMUM
SCHEDULED MAINTENANCE FOS = 1.25 MINIMUM
EXTREME MAINTENANCE FOS = 1.1 MINIMUM

GENERAL INFORMATION

1. FOR ABBREVIATIONS NOT LISTED, SEE ASME Y14.38 "ABBREVIATIONS AND ACRONYMS: PUBLICATION AS DISTRIBUTED BY THE AMERICAN SOCIETY OF MECHANICAL ENGINEERS (ASME).
2. DESIGN DETAILS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS OCCURRING THROUGHOUT THE PROJECT, WHETHER OR NOT THEY ARE INDIVIDUALLY CALLED OUT.
3. VERIFY FINAL OPENING DIMENSIONS IN WALLS, SLABS, AND DECKS WITH OTHER DISCIPLINE DRAWINGS PRIOR TO CONSTRUCTION OF THESE ELEMENTS.
4. FOR NUMBER, TYPE, SIZE, ARRANGEMENT, AND/OR LOCATION OF EQUIPMENT PADS, SEE OTHER DISCIPLINE DRAWINGS. COORDINATE WITH EQUIPMENT SUPPLIER PRIOR TO PLACING SLABS, WALLS AND FOUNDATIONS. COORDINATE PIPING OPENINGS WITH OTHER DISCIPLINE DRAWINGS.
5. DO NOT CUT OR MODIFY STRUCTURAL MEMBERS FOR PIPES, DUCTS, ETC., UNLESS SPECIFICALLY DETAILED OR APPROVED IN WRITING BY THE ENGINEER.
6. VISITS TO THE JOB SITE BY THE ENGINEER TO OBSERVE THE CONSTRUCTION DO NOT IN ANY WAY MEAN THAT ENGINEER IS GUARANTOR OF CONSTRUCTOR'S WORK, NOR RESPONSIBLE FOR THE COMPREHENSIVE OR SPECIAL INSPECTIONS, COORDINATION, SUPERVISION, OR SAFETY AT THE JOB SITE.

INSPECTION AND TESTING

1. SPECIAL INSPECTION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR INSPECTIONS REQUIRED BY THE AUTHORITY HAVING JURISDICTION. THE CONTRACTOR SHALL SCHEDULE BOTH INSPECTIONS.
2. SPECIFIED CONCRETE AND MASONRY AND OTHER MATERIAL TESTING RELATED TO SPECIAL INSPECTION DURING CONSTRUCTION WILL BE OWNER FURNISHED.
3. SPECIFIED LABORATORY TEST MIXES AND SIMILAR TEST RESULTS TO VERIFY MATERIAL QUALITY AND CONFORMANCE TO SPECIFICATIONS, AND SUBMITTED FOR REVIEW PRIOR TO ACCEPTANCE FOR USE ON THE PROJECT, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
4. SPECIAL INSPECTION AND TESTS AND STRUCTURAL OBSERVATION (OWNER FURNISHED) IS REQUIRED IN ACCORDANCE WITH CBC SECTIONS 110 AND 1704 AS INDICATED IN THE STATEMENT OF SPECIAL INSPECTIONS. REFER TO DRAWINGS [xxx] TO [xxx].

FOUNDATIONS

1. REFER TO GEOTECHNICAL DATA REPORT NO. TBD.
2. EXCAVATIONS SHALL BE SHORED TO PREVENT SUBSIDENCE AND DAMAGE TO ADJACENT EXISTING STRUCTURES, ROADS, UTILITIES, ETC.
3. FOUNDATION BEARING SURFACES SHALL BE OBSERVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF FORMWORK OR REINFORCING STEEL. THE OBSERVATION SHALL VERIFY IF THE ACTUAL EXPOSED SUBGRADE IS AS ANTICIPATED BY THE SITE SPECIFIC BORINGS, TEST PITS AND DATA REPORTS.
4. NO BACKFILL SHALL BE PLACED BEHIND WALLS UNTIL THE WALL'S CONCRETE HAS ATTAINED 100 PERCENT AND TOP SUPPORTING SLAB'S CONCRETE HAS ATTAINED 80 PERCENT OF THEIR SPECIFIED 28 DAY COMPRESSIVE STRENGTH, OR UNTIL TOP-OF-WALL FRAMING SYSTEMS, INCLUDING STEEL DIAPHRAGMS, HAVE BEEN COMPLETED.
5. NO BACKFILL SHALL BE PLACED BEHIND CANTILEVERED, FREE TOP, WALLS UNTIL THE CONCRETE HAS ATTAINED 100 PERCENT OF ITS SPECIFIED 28 DAY COMPRESSIVE STRENGTH.
6. USE OF EXPLOSIVES IS ONLY ALLOWED WITH WRITTEN PERMISSION FROM ENGINEER.

FORMWORK, SHORING, AND BRACING

1. STRUCTURES SHOWN ON THE DRAWINGS HAVE BEEN DESIGNED FOR STABILITY UNDER FINAL CONDITIONS ONLY. DESIGN SHOWN DOES NOT INCLUDE NECESSARY COMPONENTS OR EQUIPMENT FOR STABILITY OF THE STRUCTURES DURING CONSTRUCTION. CONTRACTOR IS RESPONSIBLE FOR WORK RELATING TO CONSTRUCTION ERECTION METHODS, BRACING, SHORING, RIGGING, GUYS, SCAFFOLDING, FORMWORK, AND OTHER WORK AIDS REQUIRED TO SAFELY PERFORM THE WORK SHOWN.
2. TEMPORARY SHORING SHALL REMAIN IN PLACE UNTIL ELEVATED CONCRETE FLOOR OR SLABS HAVE REACHED 80 PERCENT OF THE 28 DAY COMPRESSIVE STRENGTH AS DETERMINED BY FIELD CYLINDER BREAKS.
3. "BURY" BARS OR "CARRIER" BARS ARE NOT ALLOWED FOR THE BOTTOM MATS OF REINFORCING IN ALL ELEVATED SLABS AND ARE NOT ALLOWED FOR THE TOP MATS OF REINFORCING IN ELEVATED SLABS LESS THAN 12 INCHES THICK.

CONCRETE REINFORCING

1. REINFORCING STEEL:
TYPICAL: ASTM A615, GRADE 60
WELDED: ASTM A706, GRADE 60
2. FABRICATION AND PLACEMENT OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH CRSI "MANUAL OF STANDARD PRACTICE" AND ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE".
3. CONCRETE COVER FOR REINFORCING, UNLESS SHOWN OTHERWISE, SHALL BE:
WHEN CAST AGAINST EARTH: 3"
INTERIOR, DRY, HUMIDITY CONTROLLED AREAS:
WALLS AND SLABS: 3/4"
BEAM STIRRUPS AND COLUMN TIES: 1 1/2"
OTHER CONCRETE SURFACES: 2"
4. REFER TO WALL CORNER AND WALL INTERSECTION REINFORCING STANDARD DETAIL. WALL CORNER REINFORCING SIZES AND SPACINGS SHALL BE AS SHOWN ON THE DRAWINGS AND REFERENCED TO THIS DETAIL. TYPICAL HORIZONTAL WALL REINFORCING SHALL LAP WITH THE CORNER HORIZONTAL REINFORCING.
5. 90 DEGREE BENDS, UNLESS OTHERWISE SHOWN, SHALL BE ACI 318 STANDARD HOOKS.
6. WALL FOOTING CORNER AND INTERSECTION REINFORCEMENT BARS SHALL BE EXTENDED INTO CONNECTING FOOTINGS AND LAPPED ON THE OPPOSITE FACE OF THE CONNECTING FOOTING. OUTSIDE FACE WALL FOOTING REINFORCEMENT SHALL BE LAPPED WITH CORNER BARS. ALL WALL FOOTING REINFORCEMENT SHALL BE CONTINUOUS THROUGH COLUMNS OR PILASTERS FOOTINGS.
7. LAP VERTICAL WALL BARS WITH DOWELS FROM BASE SLABS AND EXTEND INTO TOP FACE OF ROOF SLABS AND LAP WITH TOP SLAB REINFORCEMENT. PROVIDE A MINIMUM OF FOUR FULL HEIGHT VERTICAL BARS WITH MATCHING DOWELS AT WALL ENDS, CORNERS AND INTERSECTIONS WITH SIZE TO MATCH TYPICAL VERTICAL REINFORCING STEEL SHOWN OR REQUIRED BY NOTES ABOVE.
8. LOCATE ELEVATED SLAB AND BEAM TOP BAR SPLICES AT MIDSPAN AND BOTTOM BAR SPLICES AT SUPPORTS.
9. REINFORCING STEEL FOR FOOTINGS AND SLABS ON GRADE SHALL BE ADEQUATELY SUPPORTED ON BAR SUPPORTS WITH SPACERS TO KEEP REINFORCING ABOVE THE PREPARED GRADE. LIFTING REINFORCING OFF GRADE DURING CONCRETE PLACEMENT IS NOT PERMITTED.
10. REFER TO OPENING REINFORCING STANDARD DETAILS.
11. REINFORCEMENT BENDS AND LAPS, UNLESS OTHERWISE NOTED, SHALL SATISFY THE FOLLOWING MINIMUM REQUIREMENTS:

CONCRETE DESIGN STRENGTH = 4,000 PSI **		GRADE 60 REINFORCING STEEL								
BAR SIZE		#3	#4	#5	#6	#7	#8	#9	#10	#11
LAP SPLICE LENGTH ***	TOP BAR *	1'-4"	1'-8"	2'-1"	3'-0"	5'-2"	6'-8"	8'-6"	10'-10"	13'-4"
	OTHER BAR	1'-4"	1'-4"	1'-8"	2'-4"	4'-0"	5'-2"	6'-7"	8'-4"	10'-3"
SPACING = 3"	TOP BAR *	1'-4"	1'-8"	2'-0"	2'-5"	3'-10"	5'-0"	6'-5"	8'-1"	10'-0"
	OTHER BAR	1'-4"	1'-4"	1'-7"	1'-10"	3'-0"	3'-11"	4'-11"	6'-3"	7'-8"
SPACING = 4"	TOP BAR *	1'-4"	1'-8"	2'-0"	2'-5"	3'-6"	4'-0"	5'-0"	6'-2"	7'-5"
	OTHER BAR	1'-4"	1'-4"	1'-7"	1'-10"	2'-9"	3'-1"	3'-10"	4'-9"	5'-8"
SPACING ≥ 6"	TOP BAR *	1'-0"	1'-3"	1'-8"	2'-4"	4'-0"	5'-2"	6'-7"	8'-4"	10'-3"
	OTHER BAR	1'-0"	1'-0"	1'-3"	1'-10"	3'-1"	4'-0"	5'-1"	6'-5"	7'-11"
SPACING = 4"	TOP BAR *	1'-0"	1'-3"	1'-7"	1'-10"	3'-0"	3'-11"	4'-11"	6'-3"	7'-8"
	OTHER BAR	1'-0"	1'-0"	1'-3"	1'-5"	2'-4"	3'-0"	3'-10"	4'-10"	5'-11"
SPACING ≥ 6"	TOP BAR *	1'-0"	1'-3"	1'-7"	1'-10"	2'-9"	3'-1"	3'-10"	4'-9"	5'-8"
	OTHER BAR	1'-0"	1'-0"	1'-3"	1'-5"	2'-1"	2'-5"	3'-0"	3'-8"	4'-5"

* TOP BARS SHALL BE DEFINED AS ANY HORIZONTAL BARS PLACED SUCH THAT MORE THAN 12" OF CONCRETE IS CAST IN THE MEMBER BELOW THE BAR IN ANY SINGLE POUR. HORIZONTAL WALL BARS ARE CONSIDERED TOP BARS.
 ** WHERE 3,000 PSI CONCRETE IS USED, INCREASE ABOVE LENGTHS BY 16%.
 *** LAP LENGTHS ARE BASED ON MINIMUM CONCRETE COVER OF 2". LONGER LENGTHS ARE REQUIRED FOR CONCRETE COVER LESS THAN 2".

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PRELIMINARY - NOT FOR CONSTRUCTION

DESIGNED BY: J. KELLOGG
 DRAWN BY: S. METCALF
 CHECKED BY: H. HENRIKSON
 IN CHARGE: P. RUDE
 DATE: 12-19-2023



REGISTERED PROFESSIONAL ENGINEER
 JEREMY KELLOGG
 5698 CALIFORNIA



SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING
 GENERAL STRUCTURAL NOTES 1

VERIFY SCALES
 BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.
 0" 1"
 DRAWING NO.
 MPG-0001-G-0301
 SHT 6 OF 20

REV	DATE	BY	CHK	APPR	DESCRIPTION

CAST IN PLACE CONCRETE

- 28-DAY COMPRESSIVE STRENGTHS (TO MEET STRUCTURAL STRENGTH REQUIREMENTS):
 HYDRAULIC STRUCTURES: 4,500 PSI
 BUILDING STRUCTURES: 4,000 PSI
 CONCRETE FILL: 3,500 PSI
 CURBS AND SIDEWALKS: 3,500 PSI
 DUCT BANKS AND PIPE ENCASUREMENTS
 NOT INTEGRAL WITH FOUNDATIONS: 3,500 PSI
- 56-DAY COMPRESSIVE STRENGTHS (TO MEET DURABILITY REQUIREMENTS FOR ACI 318 AND ACI 350):
 HYDRAULIC STRUCTURES: 5,000 PSI
 BUILDING STRUCTURES: 4,500 PSI
 CONCRETE FILL: 4,000 PSI
 CURBS AND SIDEWALKS: 4,000 PSI
 DUCT BANKS AND PIPE ENCASUREMENTS
 NOT INTEGRAL WITH FOUNDATIONS: 4,000 PSI
- CONTINUOUS WATERSTOP AS SPECIFIED SHALL BE INSTALLED IN ALL CONSTRUCTION JOINTS IN WALLS AND SLABS OF WATER HOLDING BASINS AND BELOW GRADE STRUCTURES UNLESS SPECIFICALLY NOTED OTHERWISE.
- CONSTRUCTION JOINTS INDICATED ARE SUGGESTED LOCATIONS. CONTRACTOR MAY REVISE LOCATION OF JOINTS, SUBJECT TO SPECIFIED REQUIREMENTS. LAYOUT SHOWING ALL CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED FOR REVIEW BY ENGINEER.
- ROUGHEN AND CLEAN CONSTRUCTION JOINTS IN WALLS AND SLABS AS SPECIFIED PRIOR TO PLACING ADJACENT CONCRETE, EXPOSING CLEAN AGGREGATE OF 1/4" AMPLITUDE SOLIDLY EMBEDDED IN MORTAR MIX.
- COORDINATE PLACEMENT OF OPENINGS, PIPE PENETRATIONS, CURBS, DOWELS, SLEEVES, CONDUITS, BOLTS AND INSERTS PRIOR TO PLACEMENT OF CONCRETE.
- NO ALUMINUM CONDUIT OR PRODUCTS CONTAINING ALUMINUM OR ANY OTHER MATERIAL INJURIOUS TO THE CONCRETE SHALL BE EMBEDDED IN THE CONCRETE.
- CONDUIT SHALL NOT BE PLACED PARALLEL WITH BEAM OR COLUMN REINFORCEMENT UNLESS SPECIFICALLY INDICATED IN DRAWINGS.
- PATCH FORM TIE HOLES IN ACCORDANCE WITH STANDARD DETAILS.

CONCRETE UNIT MASONRY

- MASONRY WALL TYPE: SPECIAL REINFORCED WALLS.
- DESIGN COMPRESSIVE STRENGTH, f_m , OF THE FINISHED ASSEMBLY AND MATERIAL PROPERTIES SHALL BE PER THE TABLE BELOW.
- MORTAR: ASTM C270, TYPE S, HYDRATED.
- GROUT: ASTM C476 COARSE GROUT. USE OF WATER REDUCERS OR SUPERPLASTICIZERS IS NOT PERMITTED.
- CONCRETE MASONRY UNITS: ASTM C90, MEDIUM WEIGHT, LINEAR SHRINKAGE SHALL NOT EXCEED 0.065 PERCENT.
- | DESIGN COMPRESSIVE STRENGTH f_m (PSI) | UNIT STRENGTH (PSI) | GROUT STRENGTH (PSI) MIN / MAX | MORTAR PROPERTIES |
|---|---------------------|--------------------------------|-------------------|
| 2,000 | 1,900 | 2,000 / 3500 | Type S |
- PLACE COURSES IN WALLS, COLUMNS, AND PILASTERS IN RUNNING BOND PATTERN.
- PROVIDE MATCHING FOUNDATION DOWELS FOR ALL TYPICAL AND ADDITIONAL VERTICAL BARS.
- PROVIDE VERTICAL BARS AND DOWELS WITH LAP LENGTHS AS SHOWN IN DETAIL 0422-004.
- STAGGER ADJACENT LAP SPLICES BY 24 INCHES WHEN SEPARATED BY 3 INCHES OR LESS.
- PROVIDE NUMBER OF FULL HEIGHT VERTICAL BARS AT EDGES OF OPENINGS AS SHOWN IN DETAIL 0422-004.
- PROVIDE FULL HEIGHT VERTICAL BARS IN 3 CELLS AT WALL CORNERS AND INTERSECTIONS AS SHOWN IN DETAIL 0422-001.
- PROVIDE HORIZONTAL CORNER AND INTERSECTION BARS WITH LAP LENGTHS AS SHOWN IN DETAIL 0422-001.
- PROVIDE REINFORCED LINTELS ABOVE AND REINFORCED BOND BEAMS BELOW OPENINGS AS SHOWN IN DETAIL 0422-002.
- PROVIDE FULL HEIGHT VERTICAL BARS WITH MATCHING DOWELS IN CELLS ADJACENT TO OPENINGS AS SHOWN IN DETAIL 0422-002.
- GROUTING: SOLID GROUT ALL CMU WALLS.
- DO NOT PLACE CONDUIT IN CELLS CONTAINING PARALLEL REINFORCEMENT.

WELDING

- WELDS SHALL CONFORM TO AMERICAN WELDING SOCIETY (AWS), LATEST EDITION:
 D1.1. STRUCTURAL WELDING CODE – STEEL
 D1.2. STRUCTURAL WELDING CODE – ALUMINUM
 D1.3. STRUCTURAL WELDING CODE – SHEET STEEL
 D1.4. STRUCTURAL WELDING CODE – REINFORCING STEEL
 D1.6. STRUCTURAL WELDING CODE – STAINLESS STEEL
- REPAIR WELDS FOUND DEFECTIVE IN ACCORDANCE WITH AWS D1.1 CLAUSE 7.25.
- USE INTERMITTENT WELDS AND A LOW HEAT INPUT WELDING PROCESS AT FIELD WELDS OF EMBED PLATES AND ANGLES TO AVOID SPALLING OR CRACKING OF THE EXISTING CONCRETE.
- BUTT JOINT AND GROOVE WELDS SHALL BE COMPLETE JOINT PENETRATION (CJP) UNLESS INDICATED OTHERWISE.

STRUCTURAL STEEL AND METAL FABRICATIONS

- STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:
 W-SHAPES A992
 MISCELLANEOUS SHAPES INCLUDING ANGLES, CHANNELS, PLATES, ETC. A36
 SQUARE OR RECTANGULAR STEEL TUBING A500, GRADE C
 STEEL PIPE A53, GRADE B
 STAINLESS STEEL SHAPES A276
- ALUMINUM SHALL CONFORM TO THE FOLLOWING STANDARDS:
 STRUCTURAL SHAPES B308
 PLATES B209
- STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN CONFORMANCE WITH THE AISC MANUAL OF STEEL CONSTRUCTION, CURRENT EDITION, AND CURRENT OSHA STANDARDS.
- FASTENERS SHALL BE HIGH STRENGTH BOLTS CONFORMING TO THE FOLLOWING EXCEPT WHERE SPECIFICALLY INDICATED OTHERWISE:
 UNLESS SHOWN OTHERWISE F3125, GRADE A325, TYPE1
 ANCHOR BOLTS (AB)
 STAINLESS STEEL F593, AISI TYPE 304 OR 316, CONDITION CW
 STEEL F1554, GR 36
 GALVANIZED STEEL F1554, GR 36 / A153
 MACHINE BOLTS (MB) A307, GRADE B
- ITEMS TO BE EMBEDDED IN CONCRETE SHALL BE CLEAN AND FREE OF OIL, DIRT AND PAINT.
- NO HOLES OTHER THAN THOSE SPECIFICALLY DETAILED SHALL BE ALLOWED THROUGH STRUCTURAL STEEL MEMBERS. NO CUTTING OR BURNING OF STRUCTURAL STEEL IS PERMITTED WITHOUT THE WRITTEN APPROVAL OF JACOBS.
- ALL STEEL MEMBERS EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED TO ASTM A123 UNLESS NOTED OTHERWISE. MEMBERS THAT ARE WELDED AFTER GALVANIZING SHALL BE TOUCHED UP WITH A ZINC RICH COATING AFTER COMPLETIONS AND INSPECTION OF THE WELD.

OPEN WEB METAL JOIST FRAMING

- JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS OF THE AISC AND THE STEEL JOIST INSTITUTE (SJI).
- SEE ROOF FRAMING PLANS FOR DESIGN LOADS.
- LOADS INDICATED ON THE DRAWINGS ARE MINIMUM DESIGN LOADS AND SHALL NOT BE CONSTRUED TO BE ALL LOADS APPLICABLE TO THE DESIGN OF THE JOISTS. DEAD LOADS INFERRED BY THE DRAWINGS WHICH WOULD BE INCLUDED IN COMMON PRACTICE, INCLUDING EQUIPMENT LOADS AND CONSTRUCTION LOADS, SHALL BE INCLUDED IN THE DESIGN.
- VERIFY AND COORDINATE EQUIPMENT WEIGHTS, LOCATIONS, AND ATTACHMENT REQUIREMENTS PRIOR TO JOIST FABRICATION. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE VERTICAL AND LATERAL SUPPORT OF EQUIPMENT AS SPECIFIED IN SECTION 01 88 15, ANCHORAGE AND BRACING. JOIST MANUFACTURER SHALL COORDINATE AND SUPPLY ADDITIONAL DIAGONAL WEB MEMBERS AT CONCENTRATED LOAD LOCATIONS.
- JOIST SIZES AND CHORD SIZES INDICATED ON THE PLANS ARE MINIMUM ONLY. DESIGN BY THE JOIST MANUFACTURER MAY RESULT IN A LARGER SIZE. JOISTS SHALL HAVE DOUBLE ANGLE CHORDS.
- DESIGN JOIST TOP CHORD AT END OF ROOF SUB-DIAPHRAGMS AND JOISTS DESIGNATED AS DRAG STRUTS FOR ADDITIONAL AXIAL LOAD (BOTH TENSION AND COMPRESSION) AS INDICATED ON THE ROOF FRAMING PLANS.
- PROVIDE CALCULATIONS, PRODUCT DATA, MATERIAL PROPERTIES, CONNECTION DETAILS, ETC FOR ALL TYPES OF JOISTS. CALCULATIONS SHALL BE STAMPED AND SIGNED BY AN ENGINEER REGISTERED IN THE STATE OF CA.
- JOIST BRIDGING, BOTTOM CHORD BRACING, AND OTHER ACCESSORIES SHALL BE PER THE MANUFACTURER'S STANDARDS AND AS INDICATED ON THE DRAWINGS. BRACING SHALL EXTEND TO WALLS, SEE DETAIL 0521-022.
- JOISTS SHALL BE CAMBERED FOR DEAD LOAD AS REQUIRED BY SJI. PROVIDE STANDARD SJI CAMBER UNLESS NOTED OTHERWISE. JOIST CAMBER SHALL BE SHOWN ON SHOP DRAWINGS.

STEEL DECKING

- FOR DECK SIZE, GAGE, AND FASTENING CONFIGURATIONS, SEE FRAMING PLANS. FASTENING CONFIGURATIONS SHOWN ARE SPECIFIC TO THE DECK PRODUCT USED AS BASIS OF DESIGN. CONTRACTOR SHALL FASTEN THE DECKING IN ACCORDANCE WITH INSTALLED DECK MANUFACTURER'S RECOMMENDATIONS TO MEET SPECIFIED CAPACITY REQUIREMENTS.
- WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3 "STRUCTURAL WELDING CODE SHEET STEEL".
- DECKING SHALL HAVE A MINIMUM 1 1/2 INCHES BEARING ON SUPPORTS.
- DECKING SHALL BE CONTINUOUS OVER THREE SPANS MINIMUM, EXCEPT WHERE SHOWN OTHERWISE.
- LOCATE OPENINGS FOR EQUIPMENT PER OTHER DISCIPLINE DRAWINGS.

DEFERRED SUBMITTALS

- DEFERRED SUBMITTALS ARE THOSE PORTIONS OF THE DESIGN WHICH ARE NOT SUBMITTED AT THE TIME OF PERMIT APPLICATION AND WHICH ARE TO BE SUBMITTED TO THE PERMITTING AGENCY FOR ACCEPTANCE PRIOR TO INSTALLATION OF THAT PORTION OF THE WORK.
- THE FOLLOWING IS A LIST OF DEFERRED SUBMITTALS THAT ARE EXPECTED TO CONTAIN STRUCTURAL CALCULATIONS OR SAFETY RELATED SYSTEM INFORMATION FOR REVIEW TO MEET BUILDING PERMITTING REQUIREMENTS FOR DESIGNED SYSTEMS. PRIOR TO INSTALLATION OF THE INDICATED STRUCTURAL ELEMENT, EQUIPMENT, DISTRIBUTION SYSTEM, OR COMPONENT OR ITS ANCHORAGE, SUBMIT THE REQUIRED CALCULATIONS AND SUPPORTING DATA AND DRAWINGS FOR REVIEW AND ACCEPTANCE.

SPECIFICATION SECTION	ITEM
01 88 15	ANCHORAGE AND BRACING
05 21 19	OPEN WEB STEEL JOIST FRAMING
33 16 13.12	BOLTED STEEL STORAGE TANK
40 05 15	PIPING SUPPORT SYSTEMS
OTHER	ANY EQUIPMENT OR COMPONENT IN WHICH A TECHNICAL SPECIFICATION REQUIRES SUBMITTAL OF EQUIPMENT OR ANCHORAGE SYSTEM CALCULATIONS

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DESIGNED BY:	J. KELLOGG
DRAWN BY:	S. METCALF
CHECKED BY:	H. HENRIKSON
IN CHARGE:	P. RUDE
DATE:	12-19-2023



REGISTERED PROFESSIONAL ENGINEER
 JEREMY KELLOGG
 5698 CALIFORNIA



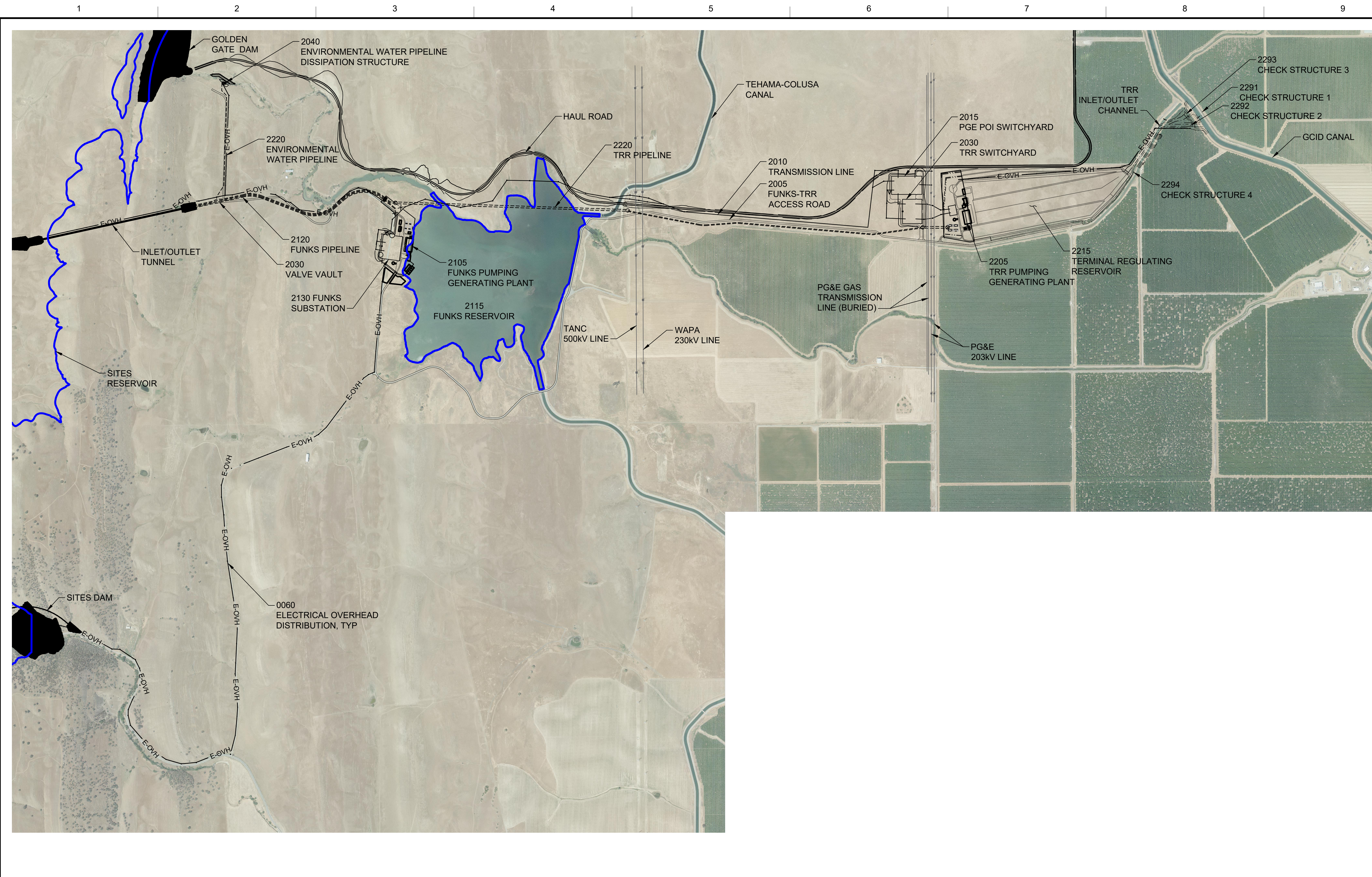
SITES RESERVOIR

MAXWELL / SITES PUMPING AND GENERATING
 GENERAL
 STRUCTURAL NOTES 2

VERIFY SCALES
BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.
0 1"
DRAWING NO.
MPG-001-G-0302
SHT 7 OF 20

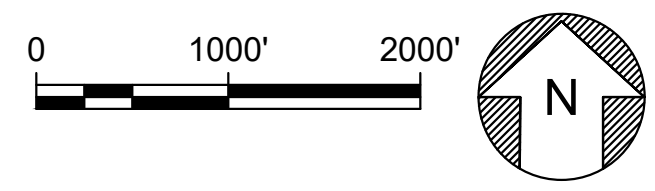
PRELIMINARY - NOT FOR CONSTRUCTION

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- ### GENERAL NOTES
1. AERIAL PHOTOGRAPHY WAS FLOWN ON MAY 13 AND 14, 2022 BY GEOTERRA, INC. AND WAS DELIVERED TO JACOBS IN SEPTEMBER 2022.
 2. MAPPING WAS COMPILED BY R.E.Y. ENGINEERS, INC. FROM AERIAL LIDAR DATA, COLLECTED BY GEOTERRA, INC. ON FEBRUARY 8 AND 9, 2022, AND SUPPLEMENTAL GROUND SURVEY AND BATHYMETRY PERFORMED BY R.E.Y. ENGINEERS.
 3. HORIZONTAL DATUM: 2011 REALIZATION OF THE NORTH AMERICAN DATUM OF 1983 (NAD83(2011)), EPOCH 2017.50. MAPPING PROJECTION IS US STATE PLANES COORDINATES, CALIFORNIA ZONE 2, SURVEY FEET.
 4. VERTICAL DATUM: NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88), GEOID18.
 5. SOURCE OF BATHYMETRY IN FUNKS RESERVOIR: R.E.Y. ENGINEERS, INC CONDUCTED THE BATHYMETRIC SURVEY IN SEPTEMBER OF 2020. DATA COLLECTION WAS BY EXTENDED RANGE-POLE WITH GPS RTK ROVER FROM A RAFT. DENSE VEGETATION IN THE RESERVOIR PREVENTED USE OF SONAR.
 6. SITES PROJECT JOINT POWERS AUTHORITY GPS CONTROL NETWORK ESTABLISHED IN JANUARY 2023. RECORD OF SURVEY IS RECORDED WITH COLUSA COUNTY RECORDS, DOCUMENT NUMBER 2023-0001608 AND WAS FILED JUNE 27, 2023.

PLAN
 HORIZ SCALE: 1" = 1000'



REV	DATE	BY	CHK.	APPR.	DESCRIPTION

DESIGNED BY:	B. CHELONIS
DRAWN BY:	B. CHELONIS
CHECKED BY:	W. OHLIN
IN CHARGE:	P. RUDE
DATE:	12-19-2023

JACOBS
 2525 AIRPARK DR
 REDDING, CA 96001
 (530) 243-5831

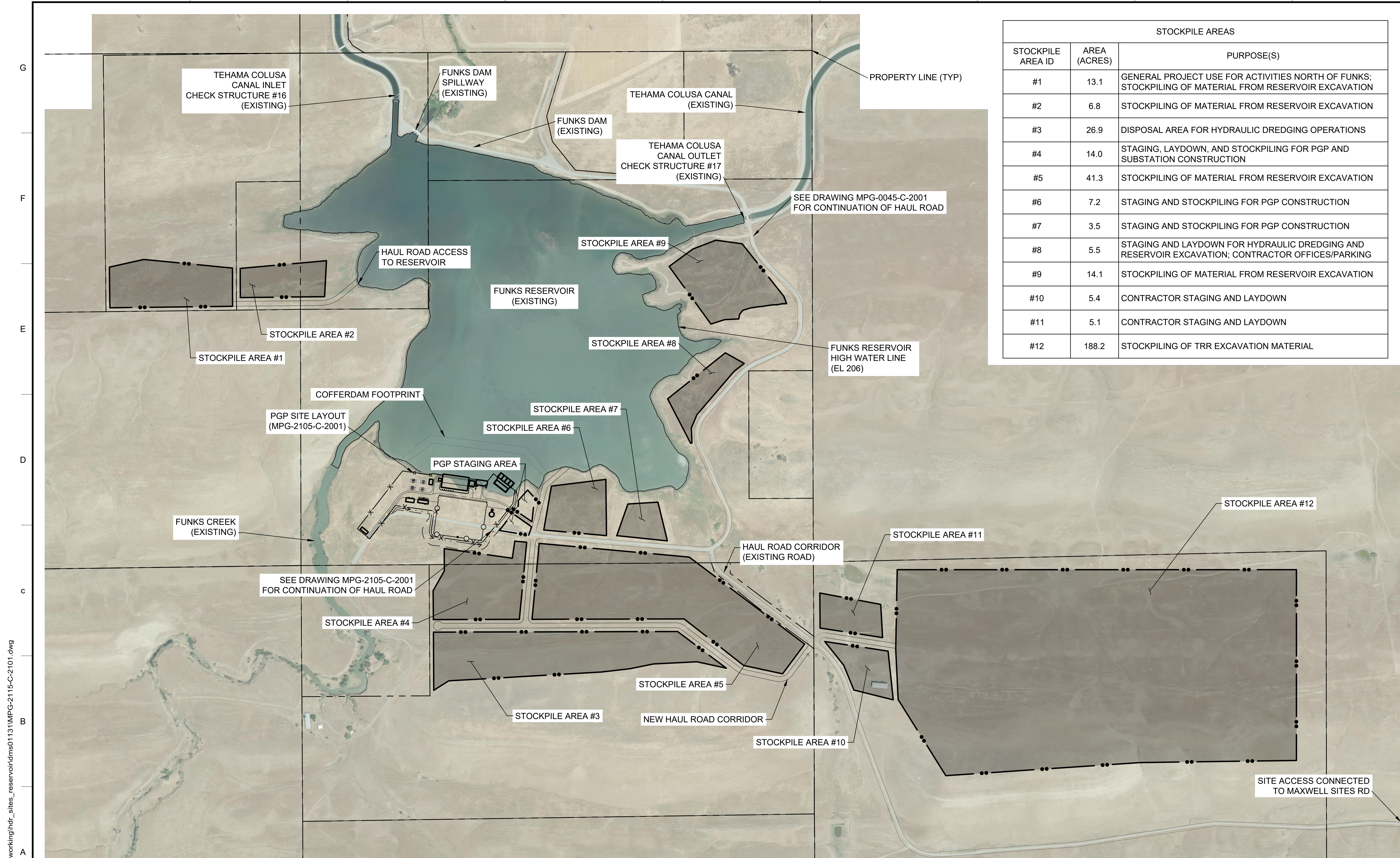
REGISTERED
 PROFESSIONAL
 ENGINEER
 BECKY K CHELONIS
 C 59851
 CALIFORNIA

SITES RESERVOIR

MAXWELL / SITES PUMPING AND GENERATING
 CIVIL
 OVERALL LOCATION
 AND SURVEY CONTROL
 PLAN

VERIFY SCALES <small>BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.</small>
DRAWING NO. MPG-0045-C-2001 SHT 8 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

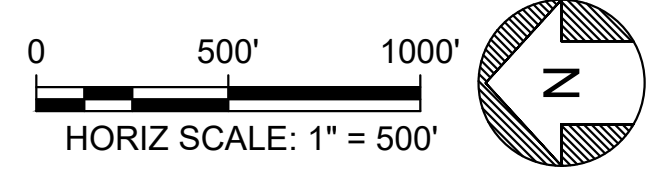


STOCKPILE AREAS		
STOCKPILE AREA ID	AREA (ACRES)	PURPOSE(S)
#1	13.1	GENERAL PROJECT USE FOR ACTIVITIES NORTH OF FUNKS; STOCKPILING OF MATERIAL FROM RESERVOIR EXCAVATION
#2	6.8	STOCKPILING OF MATERIAL FROM RESERVOIR EXCAVATION
#3	26.9	DISPOSAL AREA FOR HYDRAULIC DREDGING OPERATIONS
#4	14.0	STAGING, LAYDOWN, AND STOCKPILING FOR PGP AND SUBSTATION CONSTRUCTION
#5	41.3	STOCKPILING OF MATERIAL FROM RESERVOIR EXCAVATION
#6	7.2	STAGING AND STOCKPILING FOR PGP CONSTRUCTION
#7	3.5	STAGING AND STOCKPILING FOR PGP CONSTRUCTION
#8	5.5	STAGING AND LAYDOWN FOR HYDRAULIC DREDGING AND RESERVOIR EXCAVATION; CONTRACTOR OFFICES/PARKING
#9	14.1	STOCKPILING OF MATERIAL FROM RESERVOIR EXCAVATION
#10	5.4	CONTRACTOR STAGING AND LAYDOWN
#11	5.1	CONTRACTOR STAGING AND LAYDOWN
#12	188.2	STOCKPILING OF TRR EXCAVATION MATERIAL

GENERAL NOTES

- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.
- STOCKPILE AREAS MAY BE USED FOR CONTRACTOR STAGING AND LAYDOWN.

SHEET KEY NOTES



Plot Date: 12/18/2023 2:41 PM File: C:\pwworking\hdr_sites_reservoir\dms01131\MPG-2115-C-2101.dwg Saved By: SKHALAMEYZER

REV	DATE	BY	CHK	APPR	DESCRIPTION

DESIGNED BY:
J. GRIFFIN

DRAWN BY:
S. KHALAMEYZER

CHECKED BY:
B. MARTINEZ

IN CHARGE:
P. RUDE

DATE:
12-19-2023



REGISTERED PROFESSIONAL ENGINEER
JAY GRIFFIN
C81266
CALIFORNIA



SITES RESERVOIR
MAXWELL / SITES PUMPING AND GENERATING CIVIL
FUNKS RESERVOIR
STOCKPILE AND HAUL ROUTE PLAN

VERIFY SCALES
BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS

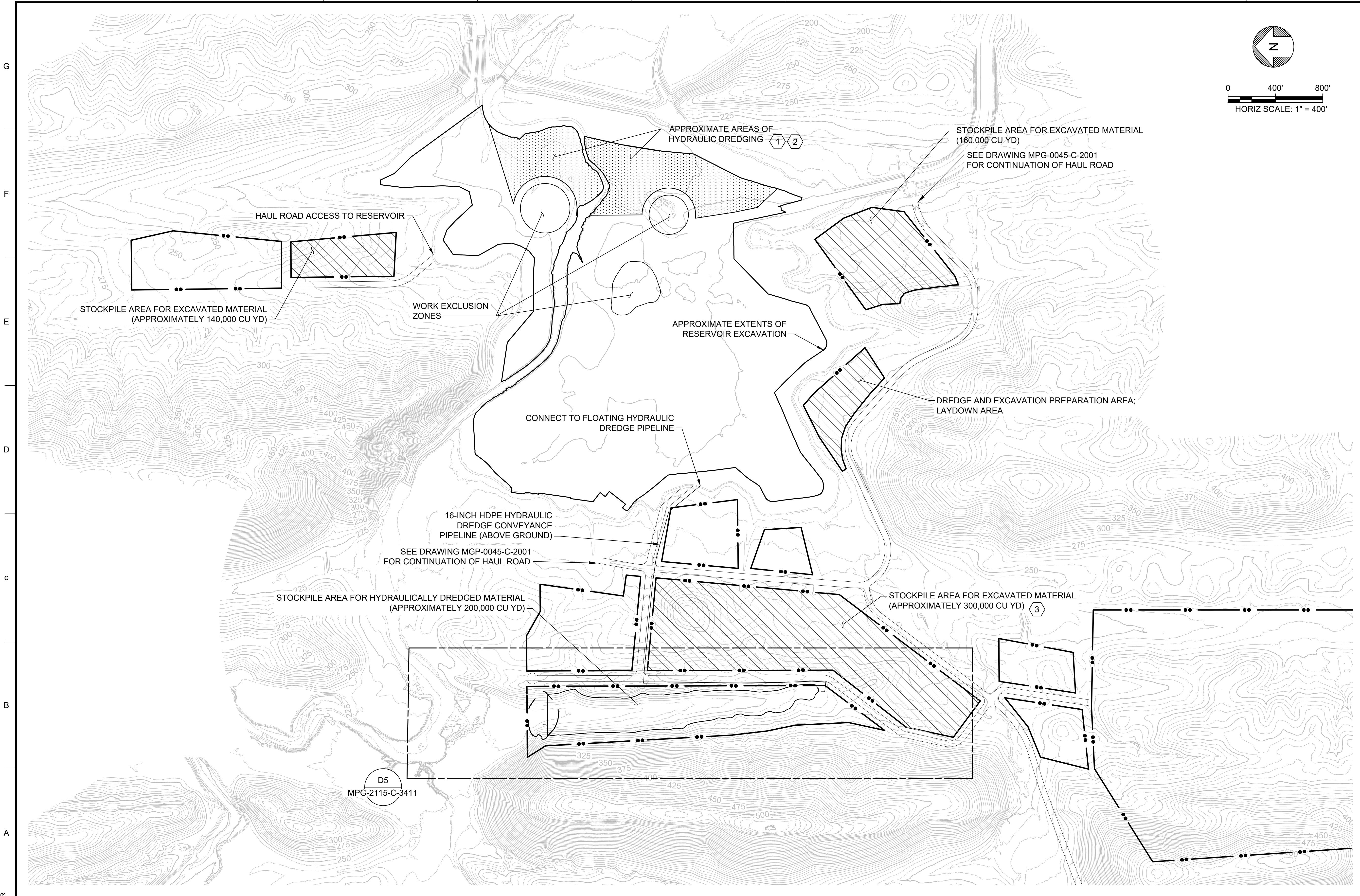
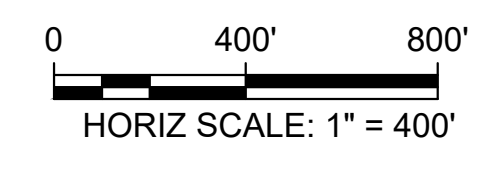
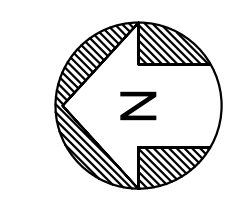
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DRAWING NO.
MPG-2115-C-2101
SHT 9 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

GENERAL NOTES

- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.



SHEET KEY NOTES

- ALL AREAS OUTSIDE OF HYDRAULIC DREDGING AREA WILL BE EXCAVATED DURING RESERVOIR OUTAGE PERIODS.
- ONE BOOSTER PUMP STATION REQUIRED ALONG THE 16-INCH HYDRAULIC DREDGE CONVEYANCE PIPELINE.
- CAPACITY INCLUDES ALLOWANCE FOR SEGREGATION OF SEDIMENTS.

Plot Date: 12/7/2023 12:15 PM File: C:\pwworking\hdr_sites_reservoir\dms01131\MPG-2115-C-2102.dwg Saved By: SKHALAMEYZER

REV	DATE	BY	CHK	APPR	DESCRIPTION

DESIGNED BY:
J. GRIFFIN

DRAWN BY:
S. KHALAMEYZER

CHECKED BY:
B. MARTINEZ

IN CHARGE:
P. RUDE

DATE:
12-19-2023



REGISTERED PROFESSIONAL ENGINEER
JAY GRIFFIN
C81266
CALIFORNIA



SITES RESERVOIR

MAXWELL / SITES PUMPING AND GENERATING
CIVIL
FUNKS RESERVOIR
RESERVOIR EXCAVATION STOCKPILES AND IMPOUNDMENT
PLAN

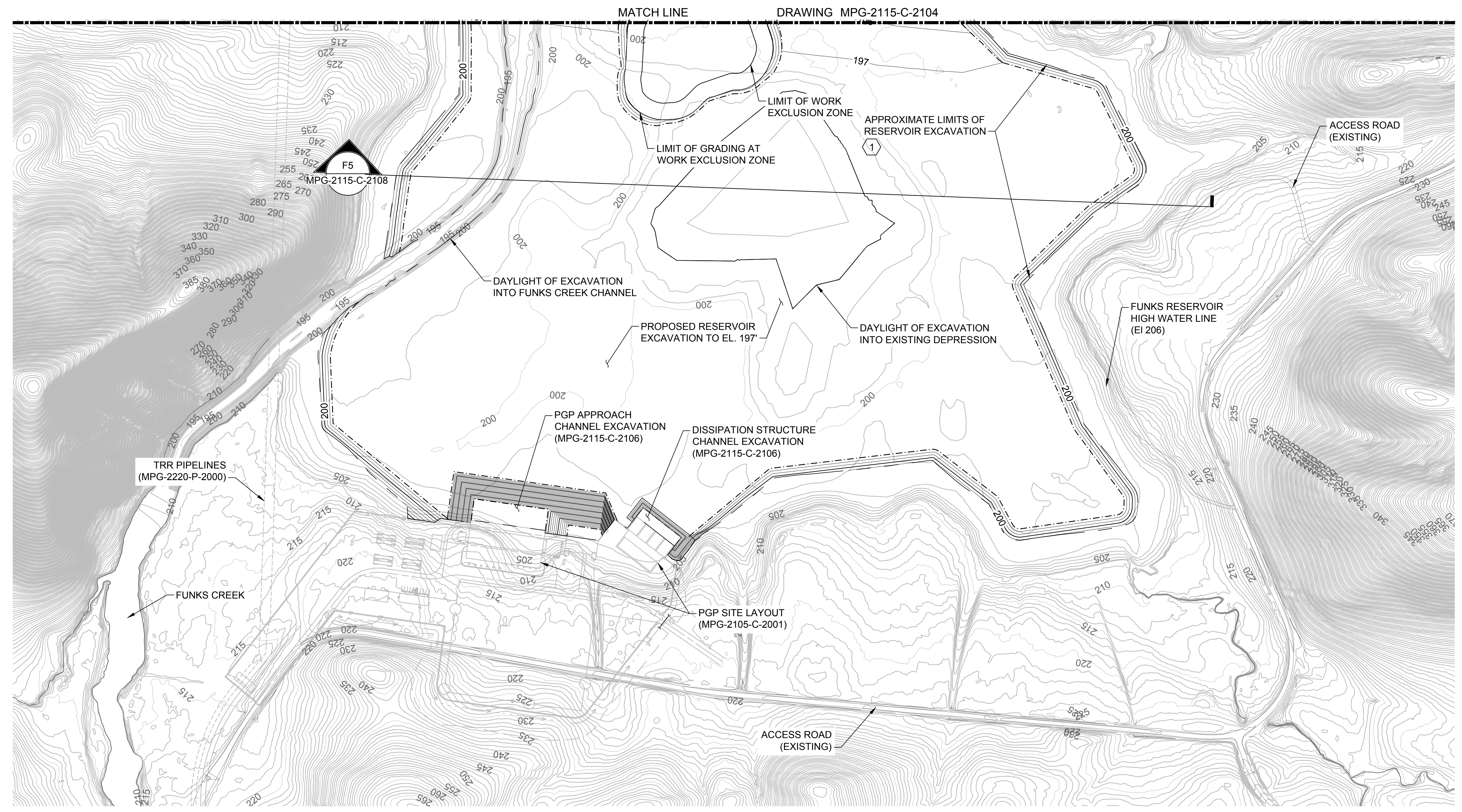
VERIFY SCALES
BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.

DRAWING NO.
MPG-2115-C-2102
SHT 10 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

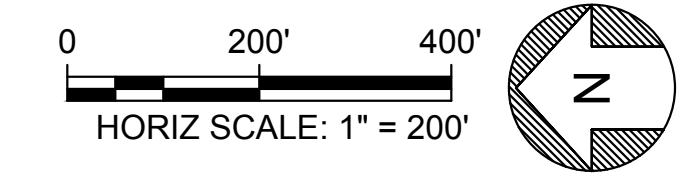
GENERAL NOTES

- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.



SHEET KEY NOTES

- RESERVOIR EXCAVATION WILL BE A MINIMUM OF 60 FT FROM WORK EXCLUSION ZONES AND APPROXIMATELY 100 FT FROM THE RESERVOIR HIGH WATER LINE.



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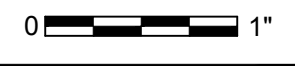
DESIGNED BY: J. GRIFFIN
 DRAWN BY: S. KHALAMEYZER
 CHECKED BY: B. MARTINEZ
 IN CHARGE: P. RUDE
 DATE: 12-19-2023


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 REDDING, CA 96001
 (530) 243-5831

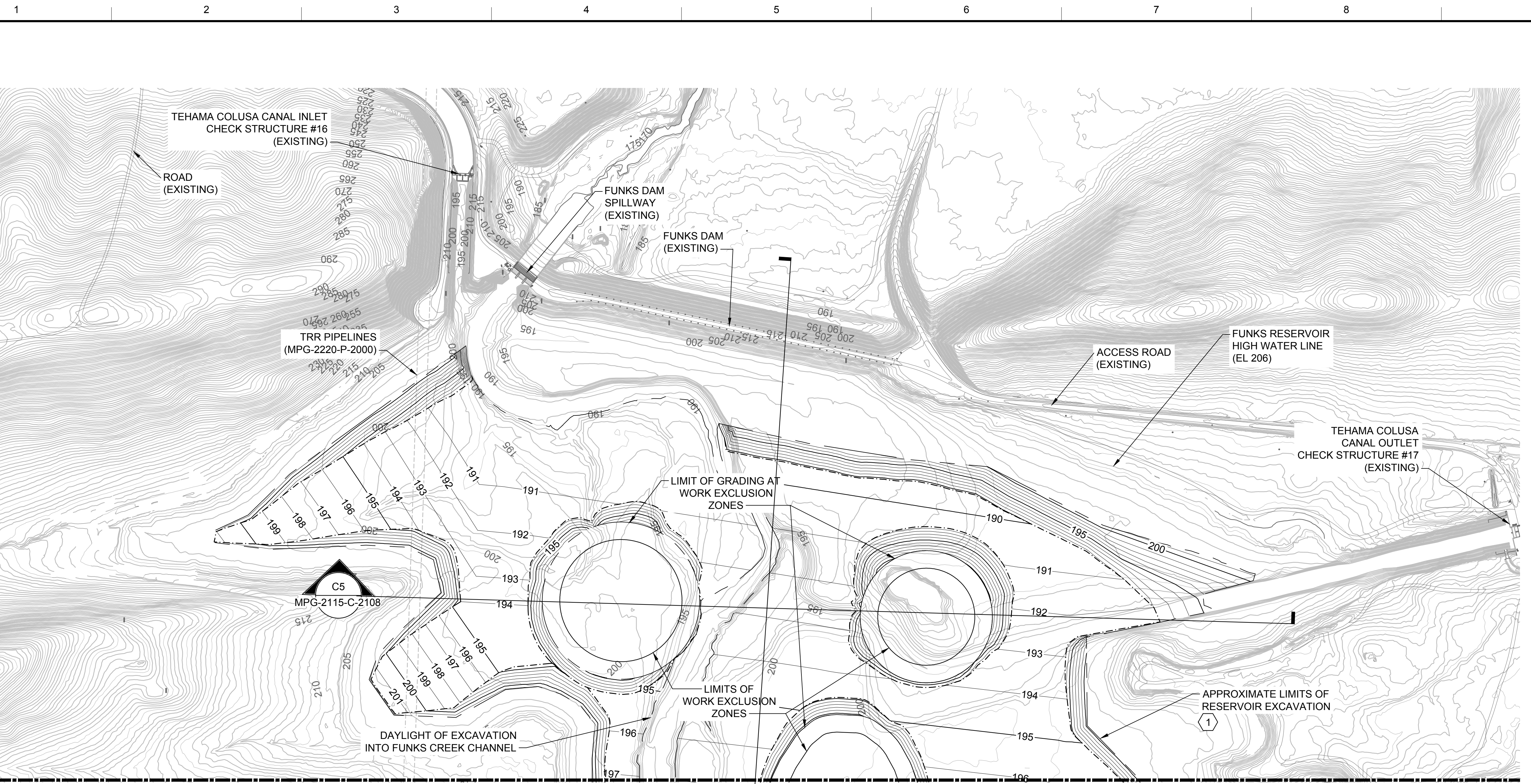

REGISTERED PROFESSIONAL ENGINEER
 JAY GRIFFIN
 C81266
 CALIFORNIA



SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING
 CIVIL
 FUNKS RESERVOIR
 WEST EXCAVATION
 PLAN

VERIFY SCALES
 BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.
 1"
 DRAWING NO.
 MPG-2115-C-2103
 SHT 11 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION



- GENERAL NOTES**
- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.
 - APPROXIMATE AREA OF HYDRAULIC DREDGING IS NOT SHOWN, SEE DWG MPG-2115-C-2102.

- SHEET KEY NOTES**
- RESERVOIR EXCAVATION WILL BE A MINIMUM OF 60 FT FROM WORK EXCLUSION ZONES AND APPROXIMATELY 100 FT FROM THE RESERVOIR HIGH WATER LINE.

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 Plot Date: 12/7/2023 2:59 PM
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REV	DATE	BY	CHK	APPR	DESCRIPTION

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 J. GRIFFIN
 DRAWN BY:
 S. KHALAMEYZER
 CHECKED BY:
 B. MARTINEZ
 IN CHARGE:
 P. RUDE
 DATE:
 12-19-2023

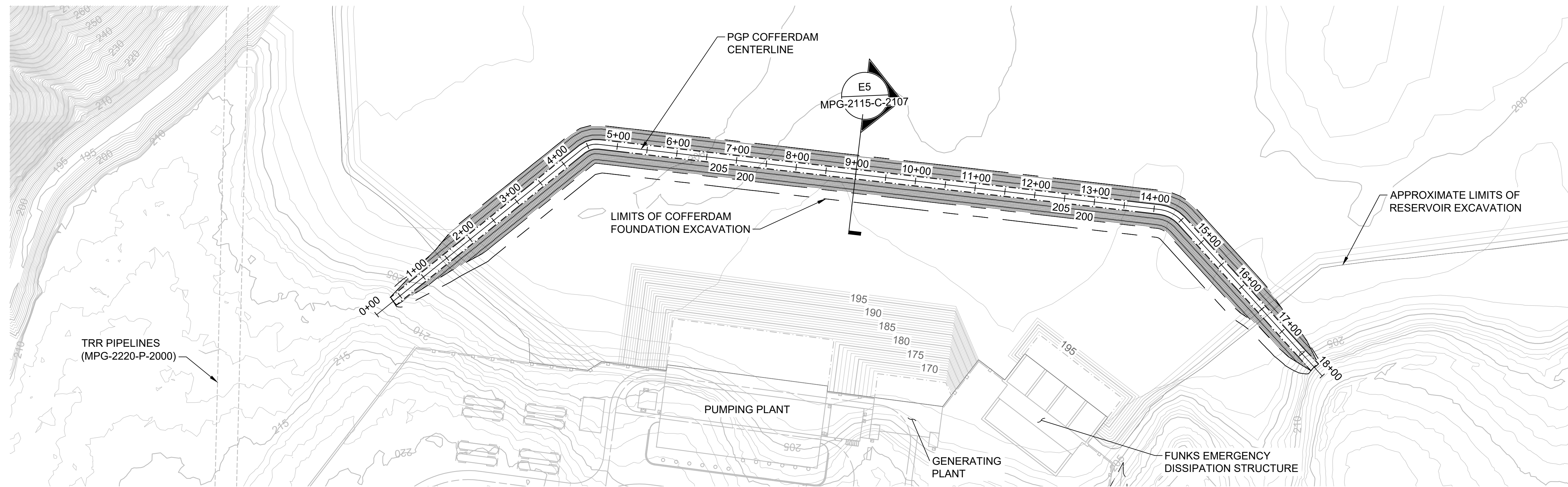
Jacobs
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 REDDING, CA 96001
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 C81266
 CALIFORNIA

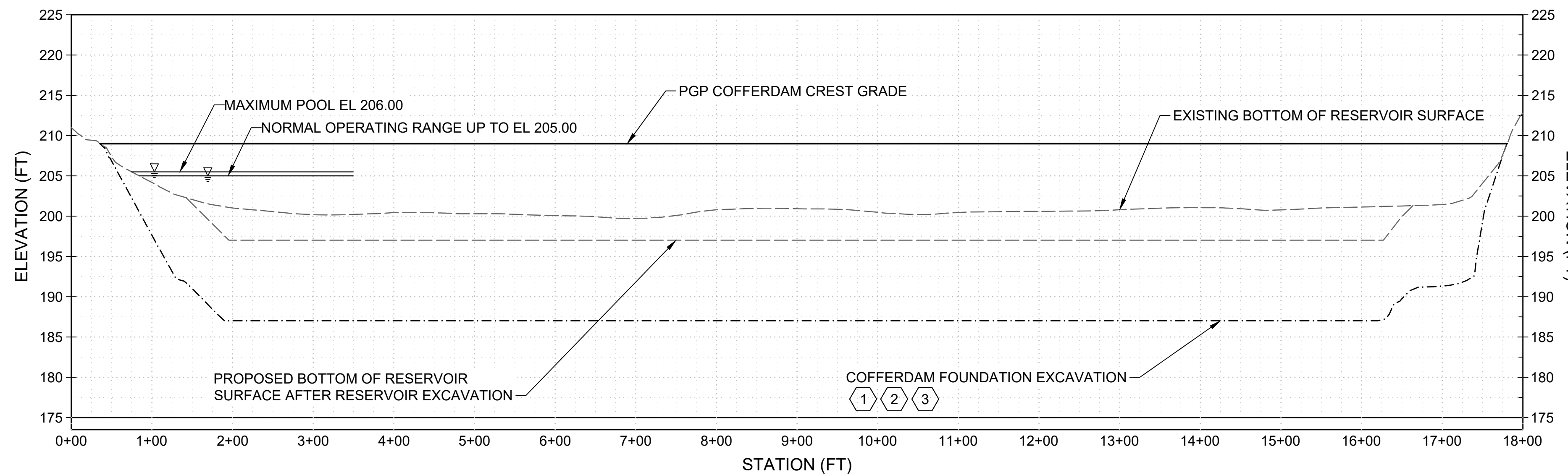
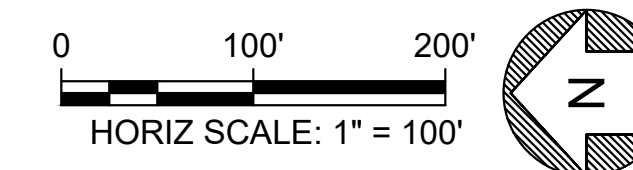
SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING
 CIVIL
**FUNKS RESERVOIR
 EAST EXCAVATION
 PLAN**

VERIFY SCALES
 BAR IS ONE INCH ON ORIGINAL
 DRAWING, ADJUST SCALES FOR
 REDUCED PLOTS
 0 200' 400'
 1"
 DRAWING NO.
 MPG-2115-C-2104
 SHT 12 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION



PGP COFFERDAM PLAN
 HORIZ SCALE: 1" = 100'



PGP COFFERDAM PROFILE
 HORIZ SCALE: 1" = 100'
 VERT SCALE: 1" = 1'

GENERAL NOTES

- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.
- FOR FUNKS PGP OVERALL SITE PLAN REFER TO DWG MPG-2105-C-2001.

SHEET KEY NOTES

- COFFERDAM FOUNDATION EXCAVATION WILL BE PERFORMED IN DRY DURING RESERVOIR OUTAGE PERIOD OR WILL BE PERFORMED VIA MECHANICAL DREDGING.
- EXTENTS AND BOTTOM ELEVATION OF COFFERDAM EXCAVATION IS APPROXIMATE AND WILL BE REVISED IN THE FIELD BASED ON ENCOUNTERED GEOLOGIC CONDITIONS.
- COFFERDAM FOUNDATION EXCAVATION SHALL BE OBSERVED BY A FIELD ENGINEER PRIOR TO BACKFILLING WITH ROCKFILL.

Plot Date: 12/18/2023 2:30 PM
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 File: C:\pwworking\hdr_sites_reservoir\dms01131\MPG-2115-C-2105.dwg

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DRAWN BY:	S.KHALAMEYZER
CHECKED BY:	B. MARTINEZ
IN CHARGE:	P. RUDE
DATE:	12-19-2023

Jacobs
 2525 AIRPARK DR
 REDDING, CA 96001
 (530) 243-5831

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 JAY GRIFFIN
 C81266
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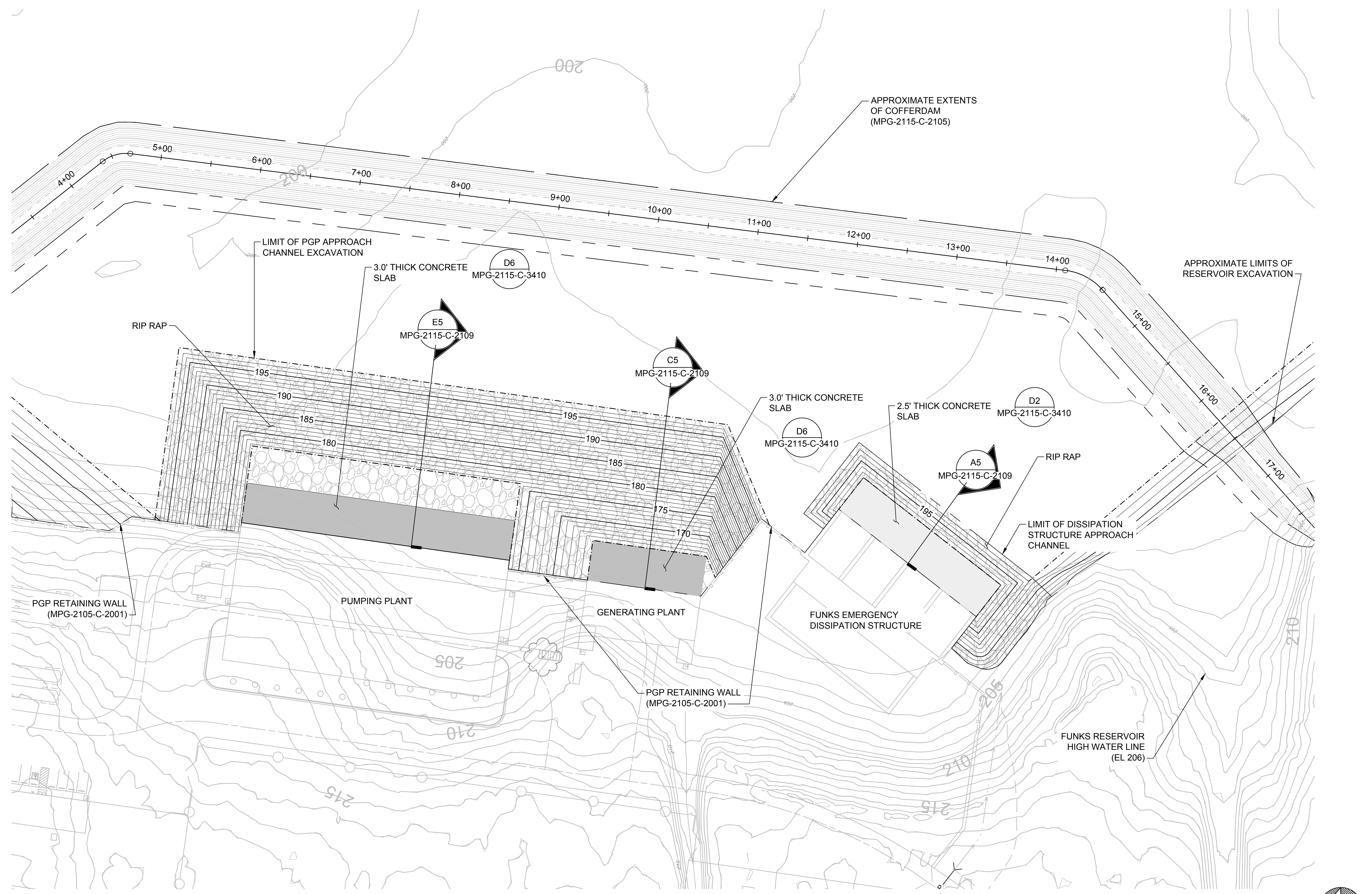
SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING CIVIL
 FUNKS RESERVOIR
 PGP COFFERDAM
 PLAN AND PROFILE

VERIFY SCALES
 BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.
 0 1" 1"

DRAWING NO.
 MPG-2115-C-2105
 SHT 13 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

- GENERAL NOTES**
- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.
 - FOR FUNKS PGP OVERALL SITE PLAN REFER TO DWG MPG-2105-C-2001.



SHEET KEY NOTES

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DESIGNED BY:	J. GRIFFIN
DRAWN BY:	S.KHALAMEYZER
CHECKED BY:	B. MARTINEZ
IN CHARGE:	P. RUDE
DATE:	12-19-2023

Jacobs
 2525 AIRPARK DR
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 (530) 243-5831

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 ENGINEER
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 C81266
 CALIFORNIA



SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING
 CIVIL
 FUNKS RESERVOIR
 PGP APPROACH CHANNEL
 PLAN

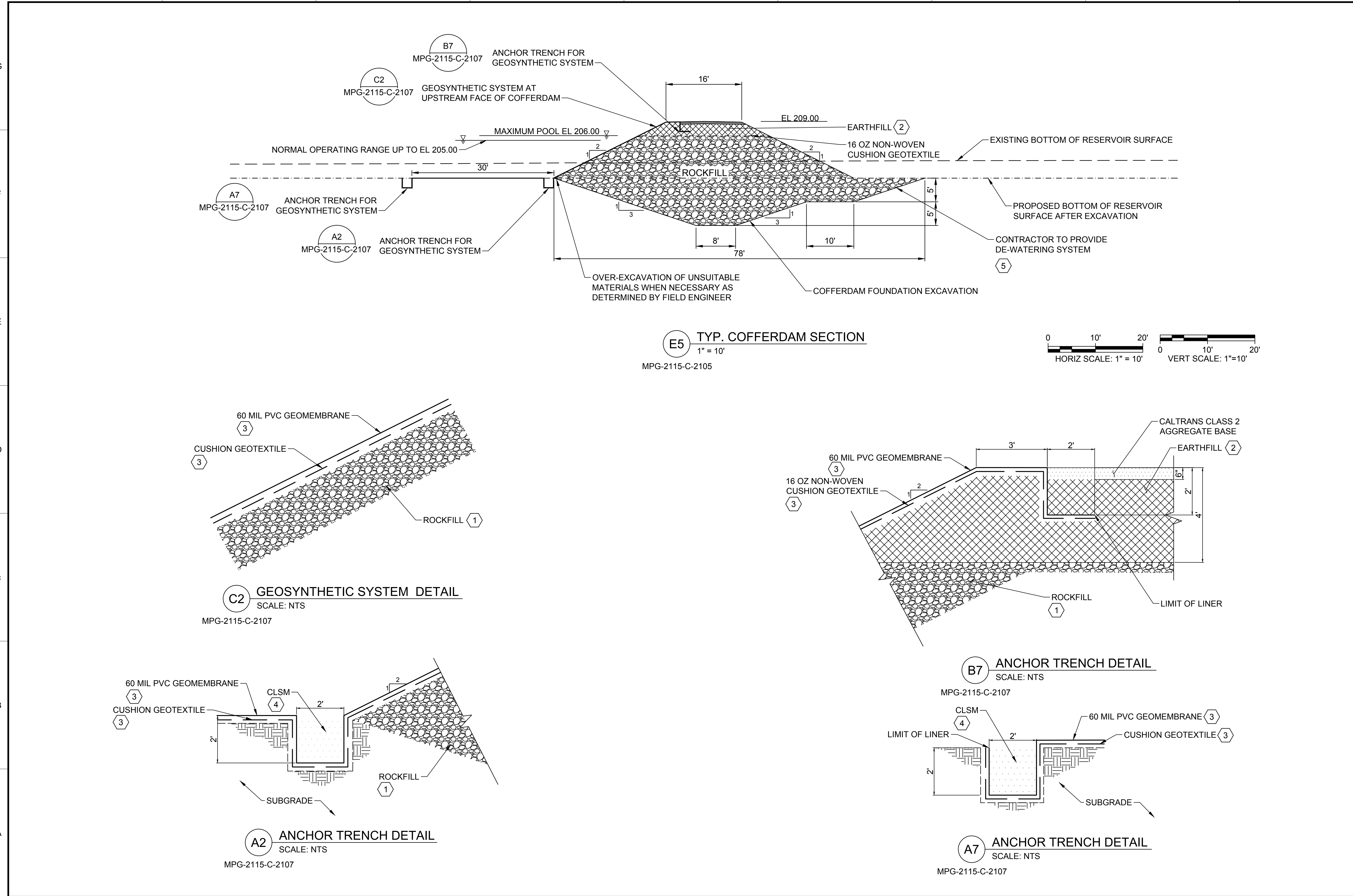
VERIFY SCALES
 BAR IS ONE INCH ON ORIGINAL
 DRAWING. ADJUST SCALES FOR
 REDUCED PLOTS

0 50' 100'
 1"

DRAWING NO.
 MPG-2115-C-2106
 SHT 14 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

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 Plot Date: 12/7/2023 6:21 PM
 Saved By: SKHALAMEYZER



- GENERAL NOTES**
- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.
 - SEE DWG MPG-2115-C-2105 FOR COFFERDAM FOUNDATION EXCAVATION NOTES.
- SHEET KEY NOTES**
- ROCKFILL SHALL BE HARD, SOUND, DURABLE, CRUSHED OR QUARRIED ROCK WITH NO PARTICLES LARGER THAN 30". ROCKFILL SHALL NOT BE SUBJECT TO DISINTEGRATION BY THE ACTION OF AIR OR WATER, AND SHALL NOT HAVE INDIVIDUAL PIECES THAT ARE ROUNDED, WORN, OR WEATHERED.
 - EARTHFILL SHALL CONSIST OF SOIL WITH A USCS CLASSIFICATION OF GW, SW, GM, GC, SM, SP, OR COMBINATION THEREOF (PER ASTM 2487), FREE FROM ORGANICS OR OTHER DELETERIOUS MATERIAL, AND SHALL NOT CONTAIN ROCKS OR LUMPS GREATER THAN 3". EARTHFILL SHALL BE PLACED IN LIFTS NOT EXCEEDING 8-INCHES AND SHALL BE COMPACTED TO A MINIMUM RELATIVE COMPACTION OF 90%.
 - GEOMEMBRANE SHALL BE 60-MIL, SMOOTH PVC, AND SHALL BE FACTORY SEAMED TO MINIMIZE FIELD SEAMING AND WASTE. GEOTEXTILE SHALL BE NONWOVEN POLYPROPYLENE WITH A MINIMUM WEIGHT OF 16 OZ/SY.
 - CLSM SHALL CONSIST OF A MIXTURE OF AGGREGATE, PORTLAND CEMENT, FLY ASH, WATER AND BENTONITE. WHEN TESTED IN ACCORDANCE WITH CALIFORNIA TEST 548, THE COMPRESSIVE STRENGTH OF A SAMPLE WILL BE AT LEAST 60 PSI AT 7 CALENDAR DAYS, AND 100 PSI TO 150 PSI AT 28 CALENDAR DAYS.
 - DEWATERING AT THE TOE OF THE COFFERDAM IS ANTICIPATED TO INCLUDE SEEPAGE COLLECTION FEATURES SUCH AS DITCHES, SUMPS, AND ASSOCIATED PUMPING EQUIPMENT TO RETURN WATER TO FUNKS RESERVOIR.

REV	DATE	BY	CHK	APPR	DESCRIPTION

DESIGNED BY: J. GRIFFIN
 DRAWN BY: S.KHALAMEYZER
 CHECKED BY: B. MARTINEZ
 IN CHARGE: P. RUDE
 DATE: 12-19-2023



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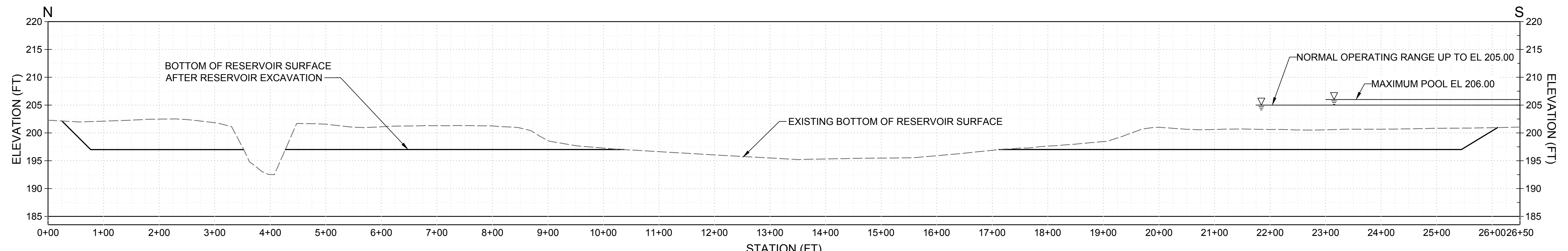


SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING
 CIVIL
 FUNKS RESERVOIR
 PGP COFFERDAM
 SECTION AND DETAILS

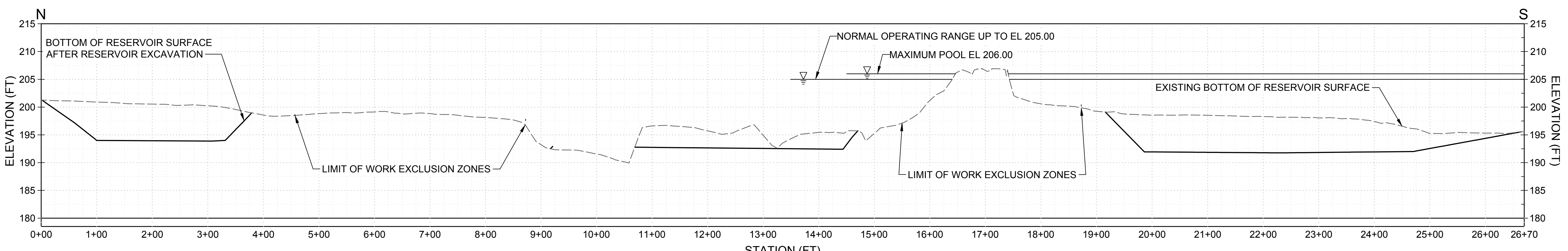
VERIFY SCALES
 BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.
 0 1"
 DRAWING NO.
 MPG-2115-C-2107
 SHT 15 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

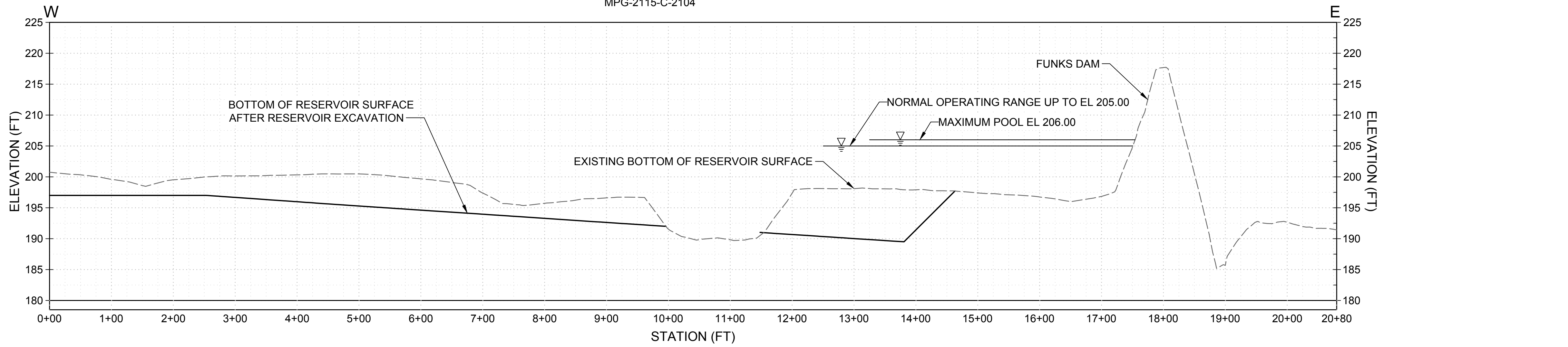
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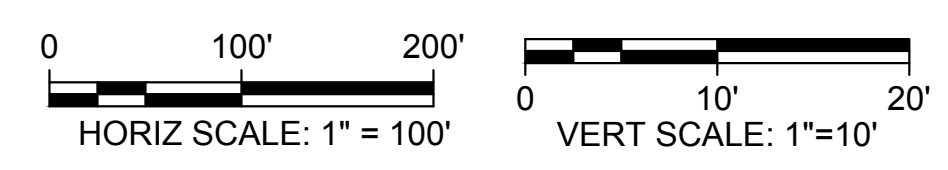
F5 RESERVOIR EXCAVATION PROFILE
 SCALE: 1" = 100'
 MPG-2115-C-2103



C5 RESERVOIR EXCAVATION PROFILE
 SCALE: 1" = 100'
 MPG-2115-C-2104



A5 RESERVOIR EXCAVATION PROFILE
 SCALE: 1" = 100'
 MPG-2115-C-2104



GENERAL NOTES

- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.
- CONSTRUCTION CONTROL TO BE GENERATED ELECTRONICALLY BY CONTRACTOR'S SURVEYOR. GRADING PLAN WILL BE PROVIDED IN ELECTRONIC FORMAT TO CONTRACTOR'S SURVEYOR UPON APPROVAL BY OWNER.

SHEET KEY NOTES

REV	DATE	BY	CHK	APPR	DESCRIPTION

DESIGNED BY: J. GRIFFIN
 DRAWN BY: S.KHALAMEYZER
 CHECKED BY: B. MARTINEZ
 IN CHARGE: P. RUDE
 DATE: 12-19-2023



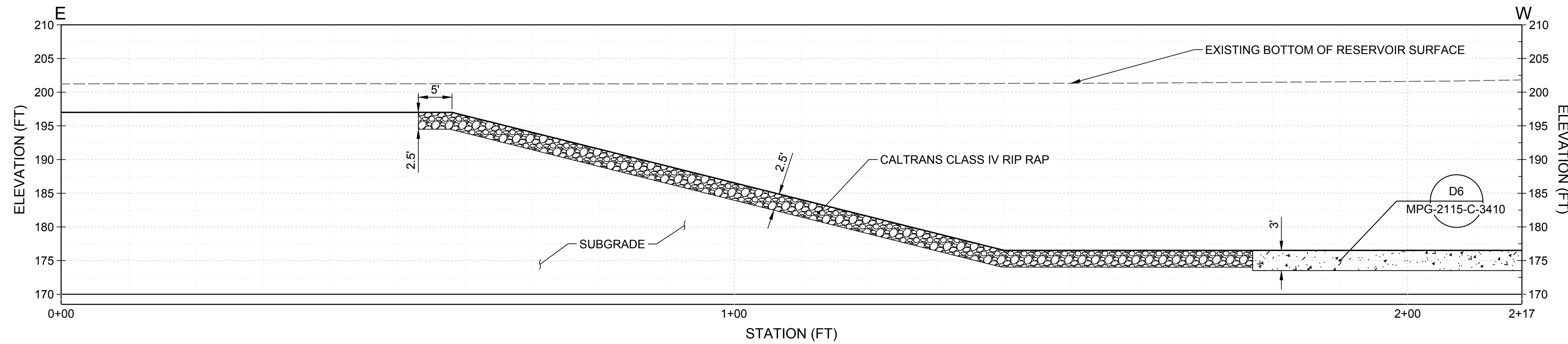
REGISTERED PROFESSIONAL ENGINEER
 JAY GRIFFIN
 C81266
 CALIFORNIA



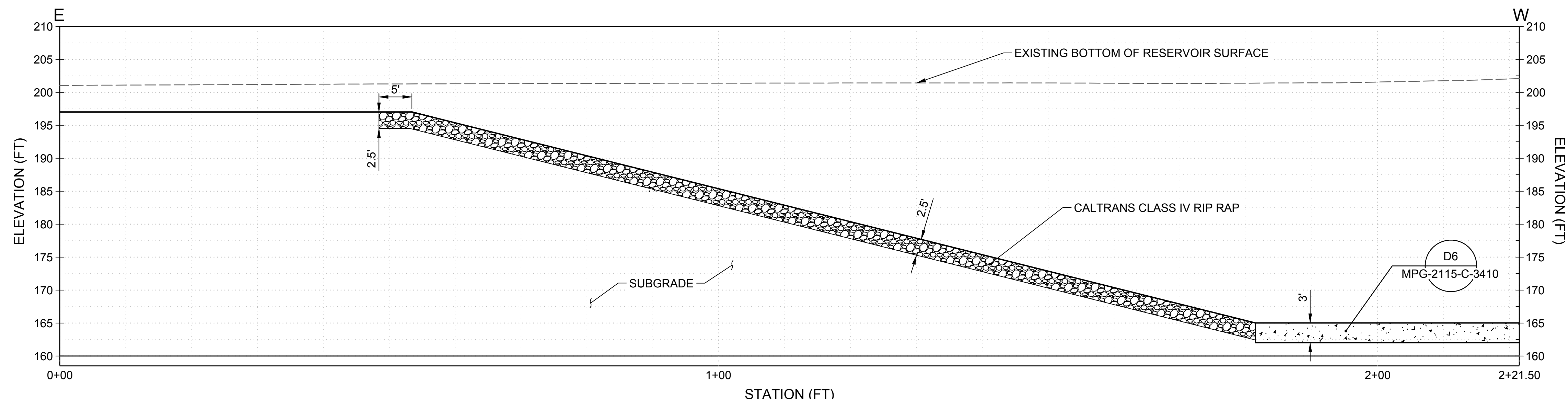
SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING CIVIL
 FUNKS RESERVOIR EXCAVATION PROFILES

VERIFY SCALES
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 DRAWING NO. MPG-2115-C-2108
 SHT 16 OF 20

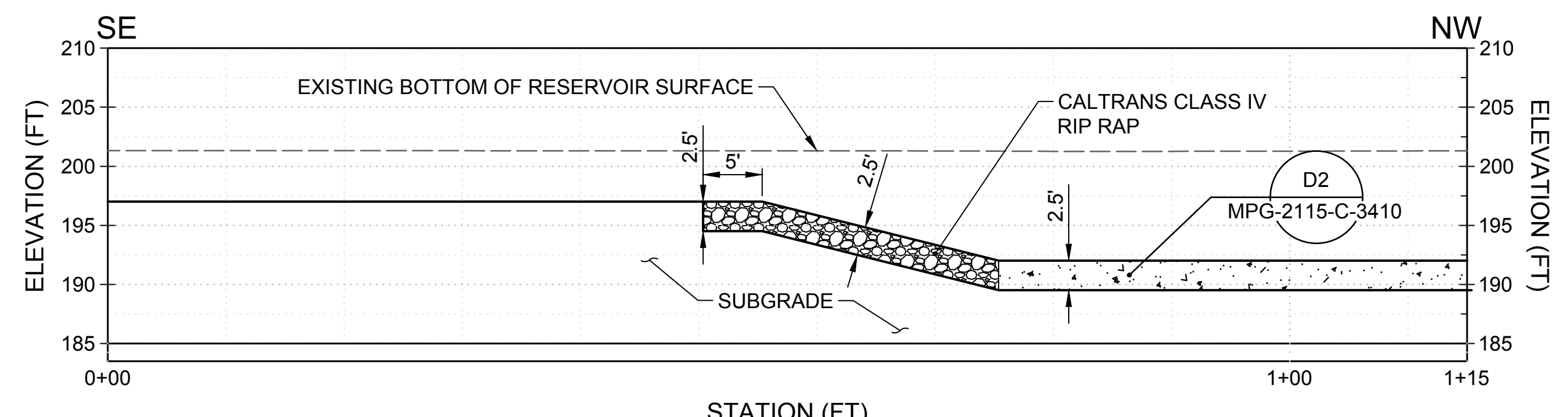
PRELIMINARY - NOT FOR CONSTRUCTION



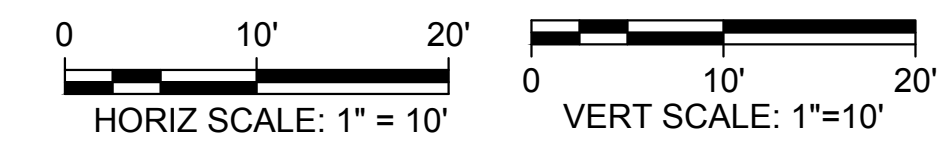
E5 APPROACH CHANNEL AT PUMPING PLANT PROFILE (TYP)
 SCALE: 1" = 10'
 MPG-2115-C-2106



C5 APPROACH CHANNEL AT GENERATING PLANT PROFILE (TYP)
 SCALE: 1" = 10'
 MPG-2115-C-2106



A5 APPROACH CHANNEL AT EMERGENCY DISSIPATION STRUCTURE PROFILE (TYP)
 SCALE: 1" = 10'
 MPG-2115-C-2106



GENERAL NOTES

- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.
- REINFORCEMENT NOT SHOWN FOR CLARITY, REFER TO DWG MPG-2115-C-3410 FOR CONCRETE DETAILS.

SHEET KEY NOTES

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REV	DATE	BY	CHK	APPR	DESCRIPTION

DESIGNED BY: J. GRIFFIN
 DRAWN BY: S. KHALAMEYZER
 CHECKED BY: B. MARTINEZ
 IN CHARGE: P. RUDE
 DATE: 12-19-2023



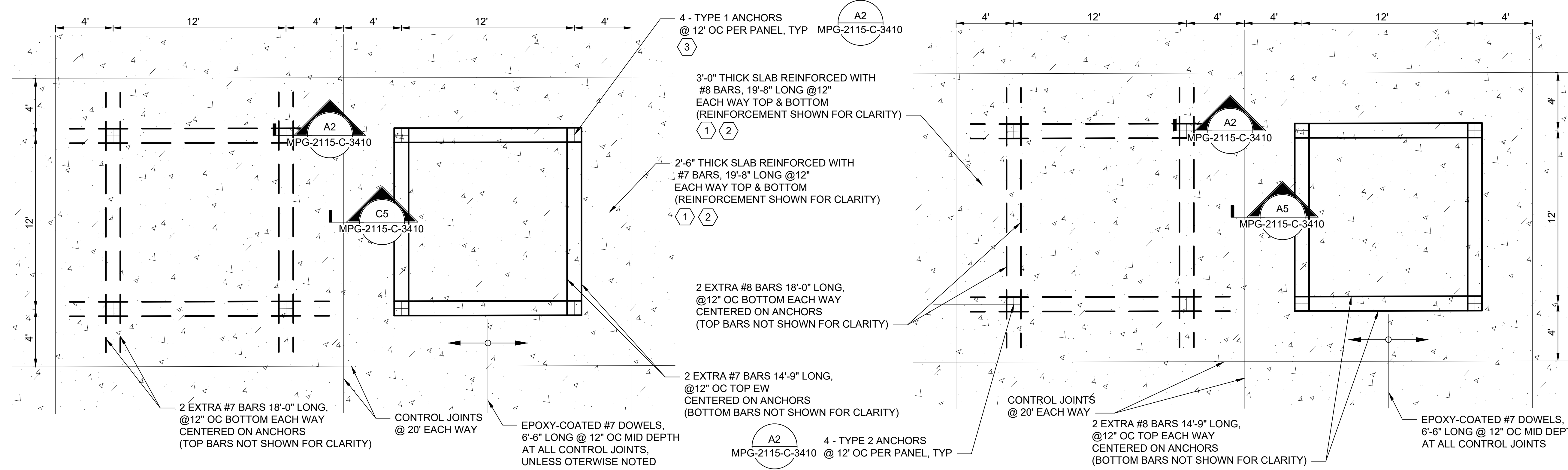
REGISTERED PROFESSIONAL ENGINEER
 JAY GRIFFIN
 C81266
 CALIFORNIA



SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING CIVIL
 FUNKS RESERVOIR
 PGP APPROACH CHANNEL PROFILES

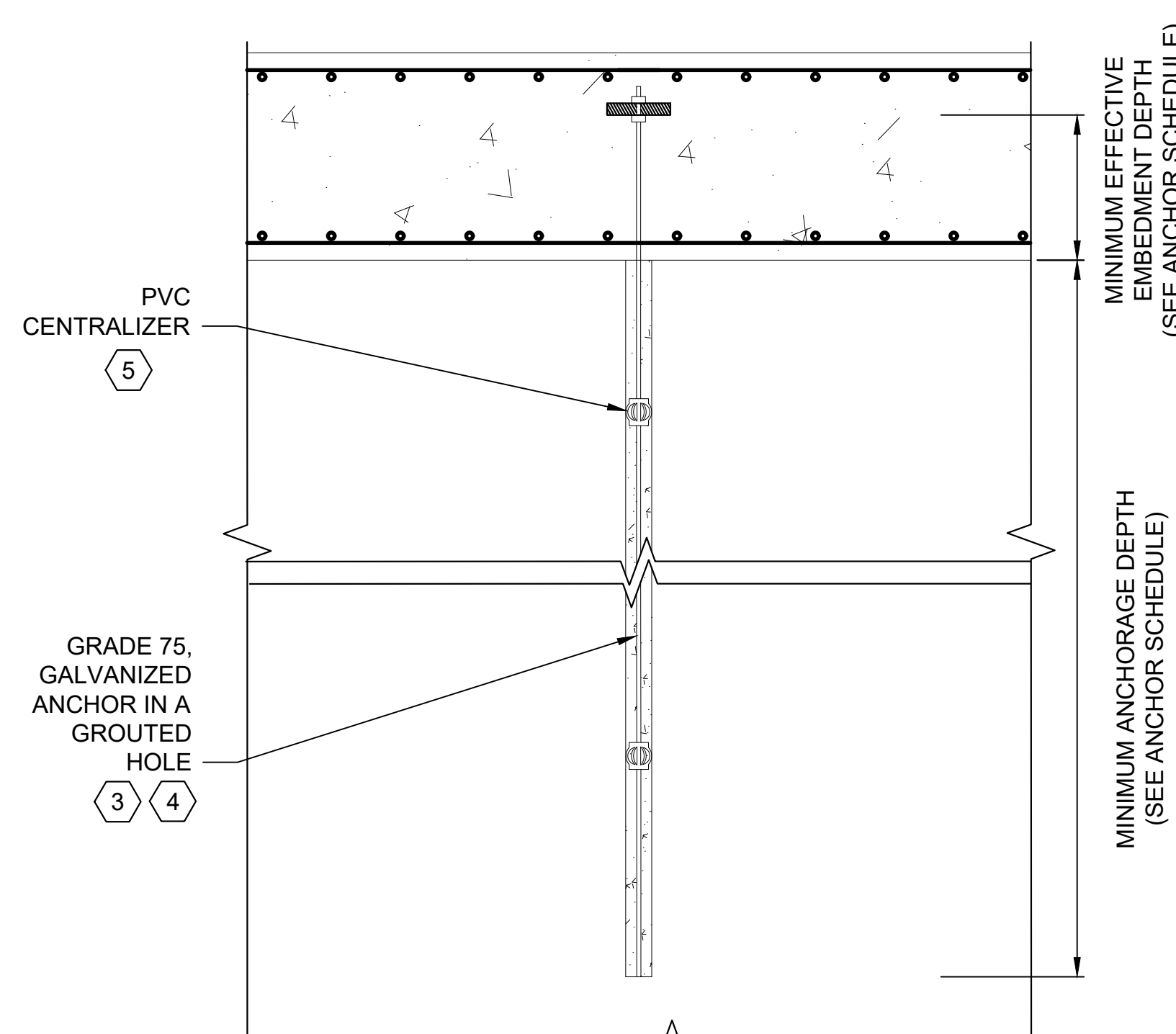
VERIFY SCALES
 BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.
 DRAWING NO. MPG-2115-C-2109
 SHT 17 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

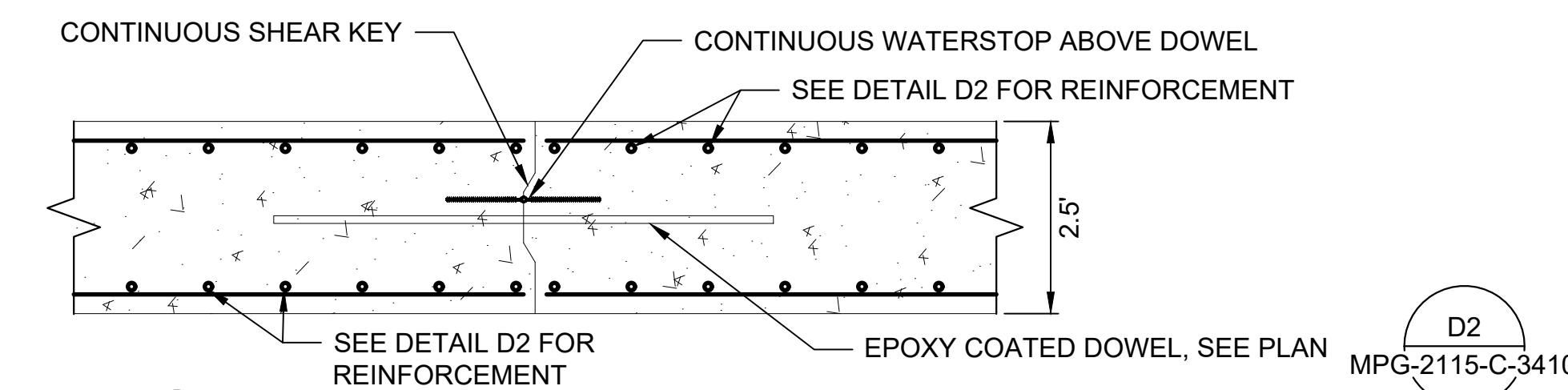


D2 2.5' THICK SLAB, CONTROL JOINT, AND ANCHOR LAYOUT
SCALE: NTS
MPG-2115-C-2106

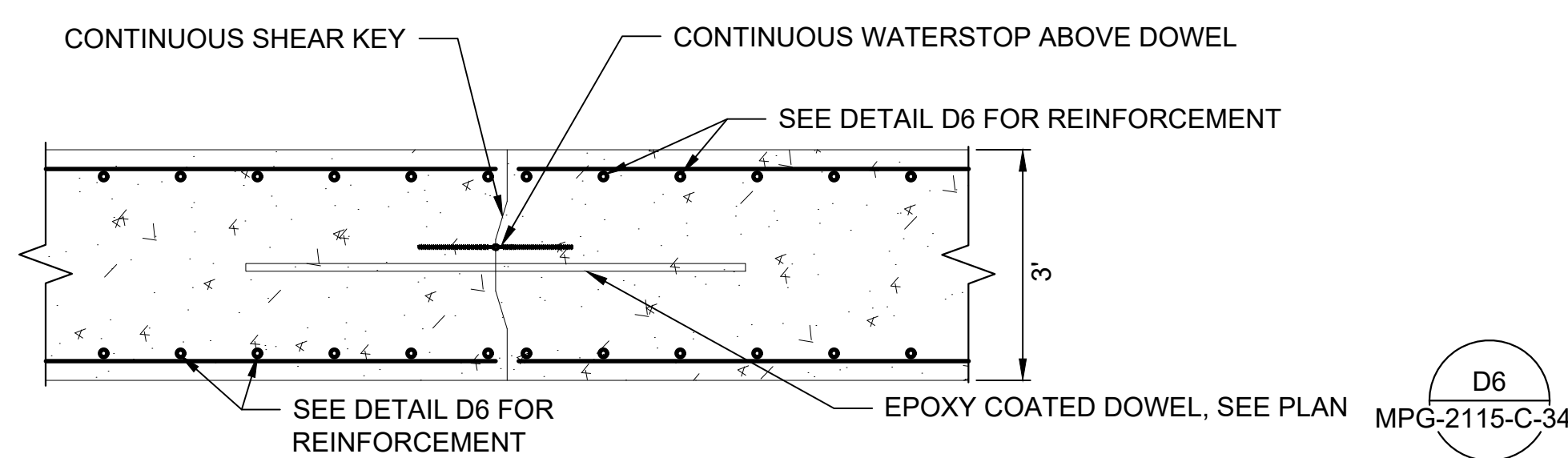
D6 3' THICK SLAB, CONTROL JOINT, AND ANCHOR LAYOUT
SCALE: NTS
MPG-2115-C-2106



A2 SLAB ANCHOR
SCALE: NTS
MPG-2115-C-3410



C5 CONTROL JOINT FOR 2.5' THICK SLAB
SCALE: NTS
MPG-2115-C-3410



A5 CONTROL JOINT FOR 3.0' THICK SLAB
SCALE: NTS
MPG-2115-C-3410

ANCHOR SCHEDULE				
TYPE	Ø	MIN. EFFECTIVE EMBEDMENT IN SLAB (FT)	MIN. ANCHOR BOND LENGTH (FT)	HEADING: BOREHOLE DIAMETER (IN)
1	#11	1'-6"	16	4
2	#14	2'-0"	16	4.5

GENERAL NOTES

1. A 3/4-INCH CHAMFER SHALL BE PROVIDED ON ALL EXPOSED CORNERS AND EDGES.
2. REINFORCEMENT SHALL BE ASTM A615 GRADE 60. 3-INCH CLEAR COVER UNLESS OTHERWISE NOTED.

SHEET KEY NOTES

1. CONCRETE SHALL BE NORMAL WEIGHT WITH A 28-DAY COMPRESSIVE STRENGTH OF 4,000 PSI.
2. PROVIDE 2 EXTRA #7 BARS AT ALL CORNERS AND EDGES FOR EACH PANEL.
3. SLAB ANCHOR BARS TO BE WILLIAMS GRADE 75, GALVANIZED FOR CORROSION PROTECTION. ANCHORS SHALL BE SUBJECT TO A PULLOUT TESTING PROGRAM PER ASTM 4435.
4. SLAB ANCHORS TO BE INSTALLED IN A WATER TIGHTNESS TESTED HOLE.
5. PVC CENTRALIZER SPACED AT 5' OC ALONG THE ENTIRE BAR LENGTH, MAXIMUM 3' FROM THE END OF THE BAR.

Plot Date: 12/18/2023 6:47 PM Saved By: SKHALAMEYZER File: C:\pwworking\hdc_sitas_reservoir\dms01131\MPG-2115-C-3410.dwg

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DRAWN BY:	S.KHALAMEYZER
CHECKED BY:	B. MARTINEZ
IN CHARGE:	P. RUDE
DATE:	12-19-2023

Jacobs
2525 AIRPARK DR
REDDING, CA 96001
(530) 243-5831

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consultants

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C81266
CALIFORNIA



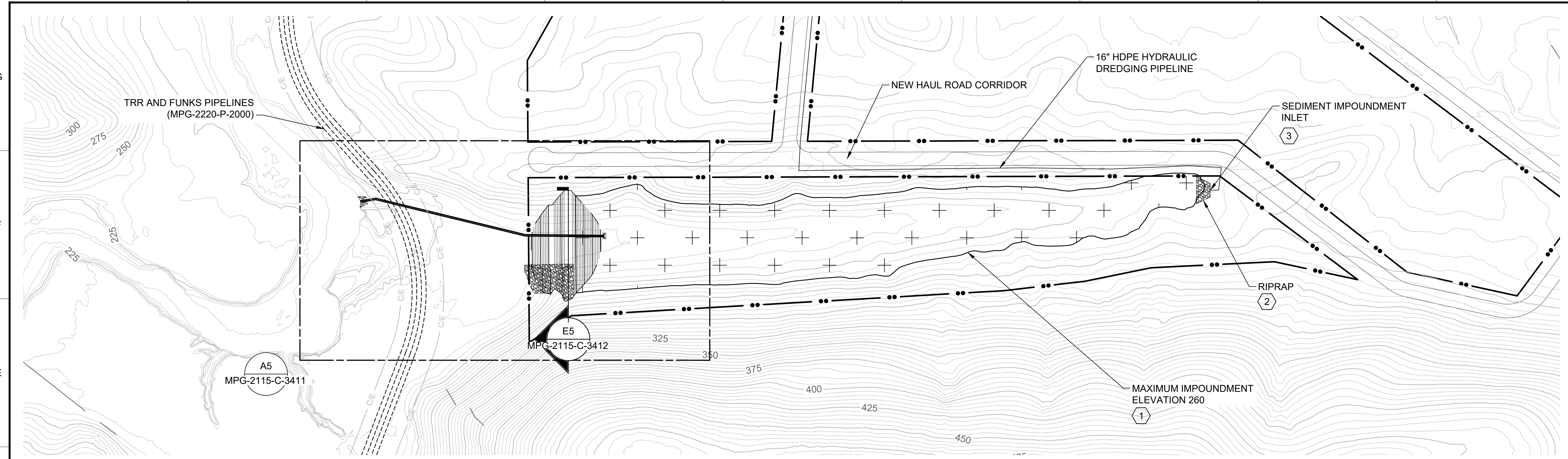
SITES RESERVOIR
MAXWELL / SITES PUMPING AND GENERATING CIVIL
FUNKS RESERVOIR
PGP APPROACH CHANNEL
DETAILS

VERIFY SCALES
BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.
0 1"

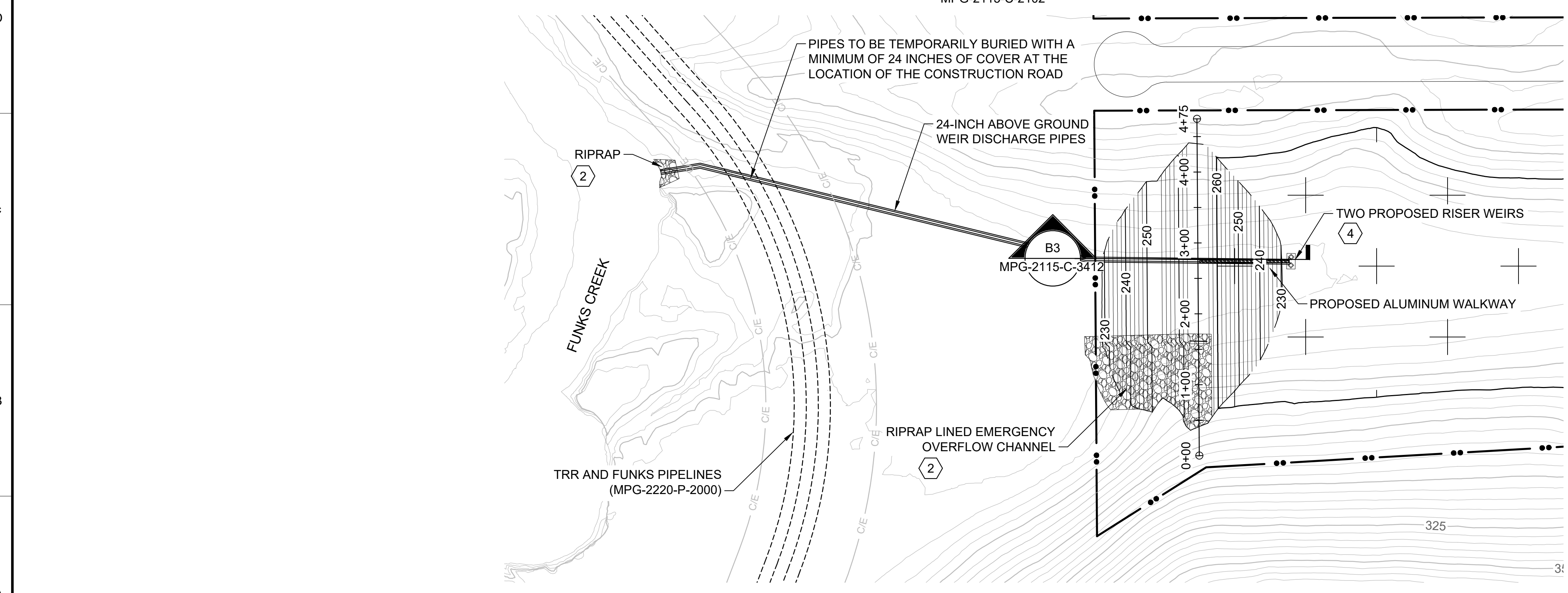
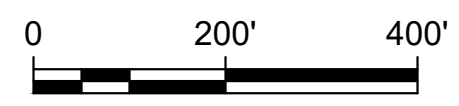
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MPG-2115-C-3410
SHT 18 OF 20

PRELIMINARY - NOT FOR CONSTRUCTION

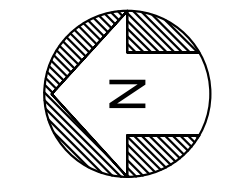
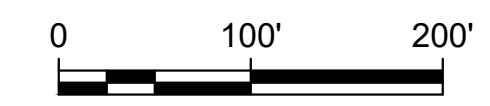
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D5 SEDIMENT IMPOUNDMENT PLAN VIEW
 HORIZ SCALE: 1" = 200'



A5 SEDIMENT IMPOUNDMENT BERM PLAN VIEW
 HORIZ SCALE: 1" = 100'



GENERAL NOTES

- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.
- KEY AND SCARIFY EXISTING MATERIAL PRIOR TO EARTHFILL PLACEMENT. PLACE EARTHFILL IN COMPACTED LIFTS WITH THICKNESS NOT EXCEEDING 6-INCHES.
- EARTHFILL WILL CONSIST OF WELL GRADED CLAYEY MATERIAL WITH A MIN. 25% PASSING THE NO. 200 SIEVE, HAVE A MAX PARTICLE SIZE OF 3 IN. AND BE FREE OF ORGANIC MATERIAL.

SHEET KEY NOTES

- CONTRACTOR TO CLEAR AND GRUB WITHIN THE EXTENTS OF THE IMPOUNDMENT.
- RIPRAP SHALL BE CALTRANS CLASS IV.
- CONTRACTOR MAY ADD FLOCCULANT OR OTHER APPROVED PRODUCTS TO ENHANCE SETTLING OF SEDIMENTS.
- RISER WEIRS TO BE STEEL FRAMED WITH SLOTS FOR 12-INCH TALL COMPOSITE WEIR BOARDS. WEIRS SHALL BE FOUNDED ON 24-INCH THICK CONCRETE SLAB AND CONNECTED TO 24-INCH HDPE OUTLET PIPES.

REV	DATE	BY	CHK	APPR	DESCRIPTION

DESIGNED BY: J. GRIFFIN
 DRAWN BY: S. KHALAMEYZER
 CHECKED BY: B. MARTINEZ
 IN CHARGE: P. RUDE
 DATE: 12-19-2023

Jacobs
 2525 AIRPARK DR
 REDDING, CA 96001
 (530) 243-5831
Geosyntec
 consultants

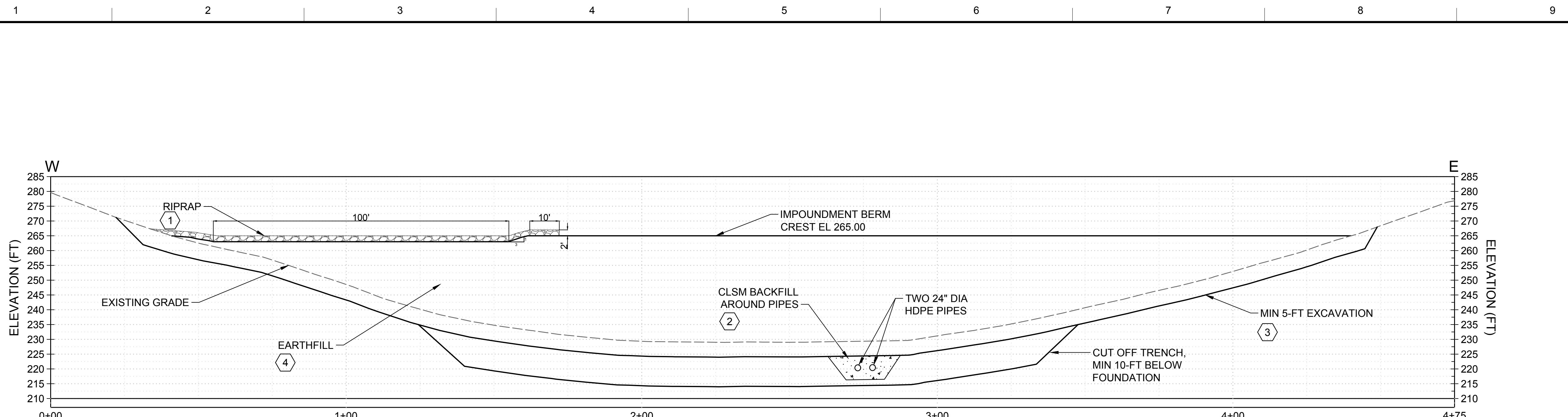
REGISTERED PROFESSIONAL ENGINEER
 JAY GRIFFIN
 C81266
 CALIFORNIA

SITES RESERVOIR
 MAXWELL / SITES PUMPING AND GENERATING CIVIL
FUNKS RESERVOIR
 SEDIMENT IMPOUNDMENT
 PLAN AND PROFILE

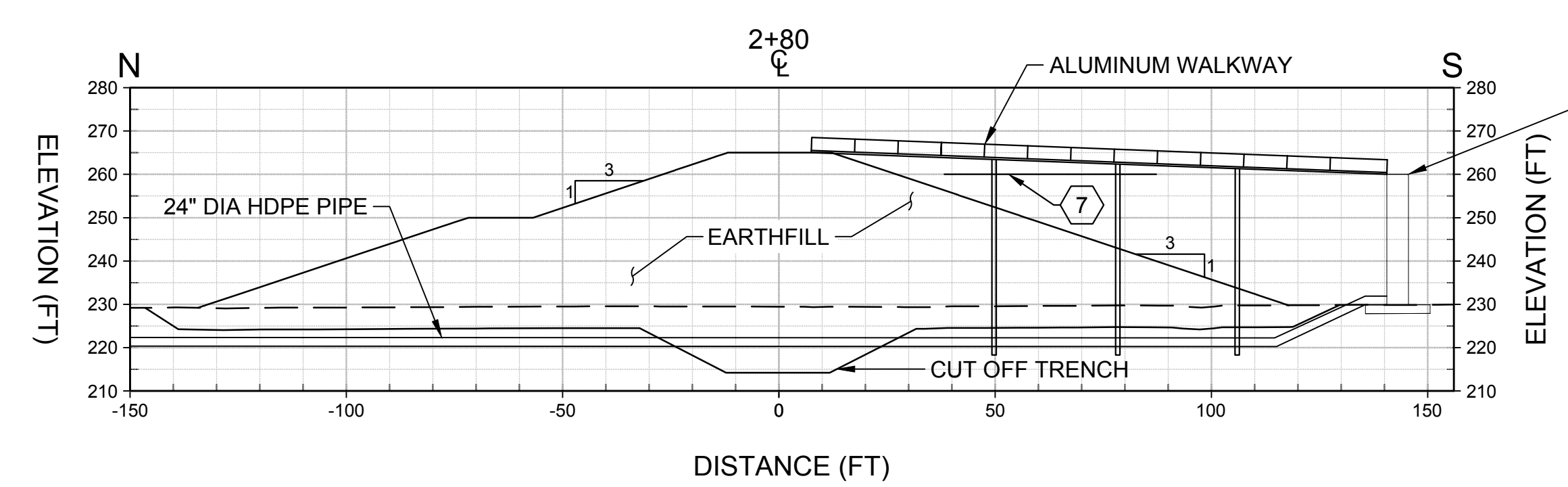
VERIFY SCALES
 BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.

 DRAWING NO.
 MPG-2115-C-3411
 SHT 19 OF 20

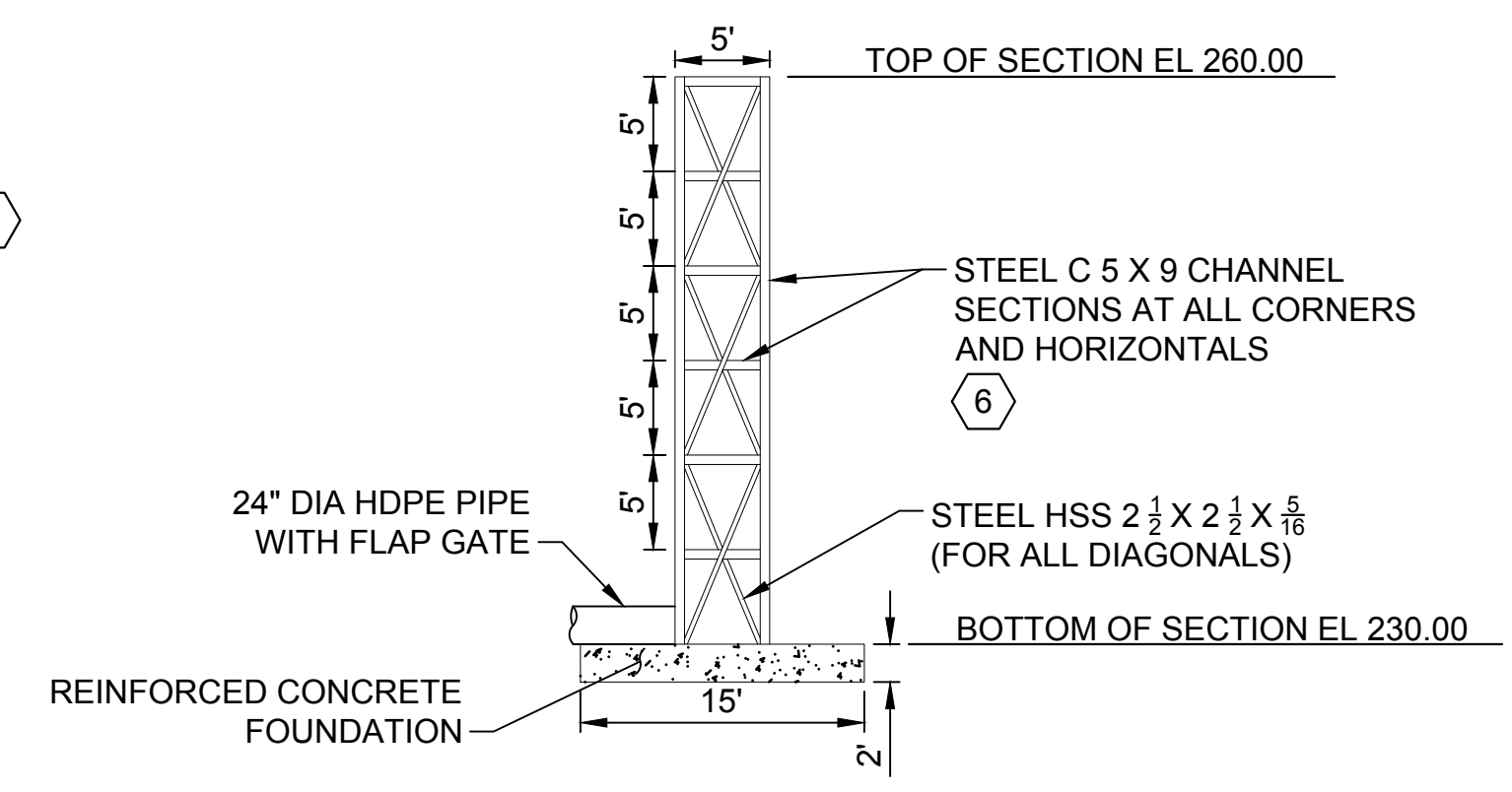
PRELIMINARY - NOT FOR CONSTRUCTION



E5 STOCKPILE #3 IMPOUNDMENT BERM PROFILE
 SCALE: 1"=20'
 MPG-2115-C-3411



B3 SECTION (TYP.)
 SCALE: 1"=30'
 MPG-2115-C-3411



B7 RISER WEIR DETAIL
 SCALE: 1"=10'
 MPG-2115-C-3412

GENERAL NOTES

- FOR SURVEY AND MAPPING NOTES REFER TO DWG MPG-0045-C-2001.
- CONTRACTOR TO CLEAR AND GRUB WITHIN THE EXTENTS OF THE IMPOUNDMENT.

SHEET KEY NOTES

- RIPRAP SHALL BE D50 = 18".
- CLSM SHALL CONSIST OF A MIXTURE OF AGGREGATE, PORTLAND CEMENT, FLY ASH, WATER AND BENTONITE. WHEN TESTED IN ACCORDANCE WITH CALIFORNIA TEST 548, THE COMPRESSIVE STRENGTH WILL BE AT LEAST 60 PSI AT 7 CALENDAR DAYS, AND 100 PSI AT 28 CALENDAR DAYS.
- KEY AND SCARIFY EXISTING MATERIAL PRIOR TO EARTHFILL PLACEMENT. PLACE EARTHFILL IN COMPACTED LIFTS WITH THICKNESS NOT EXCEEDING 6-INCHES.
- EARTHFILL WILL CONSIST OF WELL GRADED CLAYEY MATERIAL WITH A MIN 25% PASSING THE NO. 200 SIEVE, HAVE A MAX PARTICLE SIZE OF 3 IN AND BE FREE OF ORGANIC MATERIAL.
- RISER WEIRS TO BE STEEL FRAMED WITH SLOTS FOR 12-INCH TALL COMPOSITE WEIR BOARDS. WEIRS SHALL BE FOUNDED ON 24-INCH THICK CONCRETE SLAB AND CONNECTED TO 24-INCH DIA HDPE OUTLET PIPES.
- CORNERS TO INCLUDE TWO CHANNEL SECTIONS TO SERVE AS GUIDES FOR 12-INCH TALL COMPOSITE WEIR BOARDS ON ALL SIDES OF THE RISER WEIR STRUCTURES.
- MAXIMUM OPERATING ELEVATION 260 FT.

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 Plot Date: 12/7/2023 7:48 PM
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DESIGNED BY: J. GRIFFIN
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REGISTERED PROFESSIONAL ENGINEER
 JAY GRIFFIN
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 CALIFORNIA



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 PROFILE AND DETAILS

VERIFY SCALES
 BAR IS ONE INCH ON ORIGINAL DRAWING. ADJUST SCALES FOR REDUCED PLOTS.
 0 1"
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 SHT 20 OF 20

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